Taxiway A Reconstruction
ACI No. 186488
Taos Regional Airport
Taos, New Mexico

July 3, 2018

Terracon Project No. 66185006

# Prepared for:

Armstrong Consultants, Inc. Albuquerque, New Mexico

# Prepared by:

Terracon Consultants, Inc. Albuquerque, New Mexico

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Armstrong Consultants, Inc. 2305 Renard Place SE Albuquerque, New Mexico 87106

Attn: Ms. Jacquie Smith

P: (50-5) 508-2192

E: jsmith@armstrongconsultants.com

Re: Geotechnical Engineering Report

Taxiway A Reconstruction
Taos Regional Airport

ACI No. 186488 Roswell, New Mexico

Terracon Project No. 66185006

Dear Ms. Smith:

Terracon has completed geotechnical engineering exploration for the Taxiway A Reconstruction project located at the Taos Regional Airport in Taos, New Mexico. Refer to Figure A1, Site Location Map, for the project location.

The scope of the services performed for this project included site reconnaissance by a geotechnical field technician, a subsurface exploration program, and laboratory testing. These services were performed in general accordance with our proposal number P66185006 dated January 11, 2018 and the Request for Proposal (RFP) entitled "Taos Regional Airport, Taos, New Mexico - Geotech, ACI No. 186488" dated January 10, 2018, which included project information, site plan, boring location map and scope of work prepared by Armstrong Consultants, Inc. (Armstrong). The purpose of this study will be to evaluate the pertinent geotechnical conditions at the site and to assist Armstrong in the design and construction of new pavements and other site development elements.

# 1.0 PROJECT DESCRIPTION

ITEM	DESCRIPTION						
Location	Taos Regional Airport in Taos, New Mexico.						
Existing improvements	Regional airport with existing runways, taxiways, aprons, and structures.						
Existing pavement surface	Asphalt concrete						

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ITEM	DESCRIPTION
Anticipated pavement section thickness	Unknown
Current ground cover	Asphalt and Portland cement concrete pavement.
Grading	At existing taxiway grade.
Existing topography	Relatively flat to gently sloping down to the south and southwest (assumed).

# 2.0 SUBSURFACE EXPLORATION AND TESTING PROCEDURES

A total of 29 test borings were drilled at the site between the period of April 26 through May 4, 2018 using a CME-75 truck-mounted drilling rig utilizing a hollow stem auger equipped with a center bit. Sixteen (16) test borings were drilled to a depth of about 6 to 6-½ feet below existing pavement grade and 13 test borings were drilled to a depth of about 15 feet below existing site grade. Borings were performed at the locations designated by Armstrong and were referenced by station number and along taxiway or cross taxiway centerline. Refer to Figure A2, Exploration Location Map, for the boring locations. The station number are shown on each individual boring log.

Lithologic logs of the test borings were recorded by a geotechnical field technician during the drilling operations. The existing pavement materials were measured and documented for each boring location. At selected intervals designated by the RFP (below pavement, 2, 3.5, 5, and 10 feet), samples of subsurface materials and penetration tests were taken by driving split-spoon or ring-lined barrel samplers using a 140-pound hammer falling 30 inches. The number of blows required to advance the samplers the last 12 inches, or less if hard materials were present, of an 18-inch sampling interval was recorded as the standard penetration resistance value (N). An automatic SPT hammer was used to advance the split-spoon or ring-lined barrel samplers. The effect of the automatic hammer's efficiency has been considered in our interpretation and analysis.

In addition, four (4) bulk samples of the subgrade materials were obtained at the designated locations in the RFP (Boring Nos. B-02, B-13, B-21 and B-29). All samples were sealed in the field and then returned to our laboratory for testing and classification.

Groundwater measurements were made in the test borings during and at the completion of drilling.

Upon the completion of the drilling operations, the borings were backfilled with auger cuttings, compacted, and quickcrete and heated asphalt cold patch (matching the existing pavement section thickness) was placed at the top of each boring.

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# 3.0 LABORATORY TESTING PROGRAM

Selected soil samples were tested for the following engineering properties in accordance the referenced ASTM Standards:

- Gradation (ASTM D422)
- Water Content (ASTM D2216)
- Consolidation (ASTM D2435)
- Atterberg Limits (ASTM D4318)
- Dry Density (ASTM D7263)
- Moisture/Density Relationship (ASTM D1557)
- California Bearing Ratio (ASTM D1883)
- Soluble Sulfate Content

As part of the testing program, the soil samples were examined in the laboratory by the field engineer. Based on the material's texture and plasticity, the soil samples were described and classified in accordance with the attached *General Notes* and *the Unified Soil Classification System*, respectively. The estimated group symbols for the *Unified Soil Classification System* are shown in the appropriate column on the boring logs.

The AASHTO and FAA Frost Group classifications were determined for the four (4) bulk samples of the subgrade. A brief description of the Unified System is included in Appendix A.

# 4.0 SURFACE AND SUBSURFACE CONDITIONS

# 4.1 Existing Pavement Conditions

Cursory pavement observations were performed on April 26, 2018. Based on limited field observations, the existing pavement appears to be in relatively poor to fair condition with varying degrees of distress. Observed surficial pavement distress included transverse and longitudinal cracking, as well as oxidation, weathering and raveling of the pavement surface. The as-built plans and maintenance records for the pavement were not provided; however, it appeared that some crack filling of the pavement surface may have been performed. The existing pavement thicknesses at the boring locations are as follows:





			Existing Pa	vement Thick	ness (inches)	
Boring No.	Station No.	Asphalt Concrete	Portland Cement Concrete	Asphalt Treated Aggregate Base Course	Aggregate Base Course	Select Fill/ Sand and Gravel Subbase
B-01	10+00*	2	N/A	N/A	1.5	18
B-02	12+00	2.5	N/A	N/A	1.5	18
B-03	14+00*	2	N/A	N/A	1.5	18
B-04	14+00	2	N/A	N/A	1.5	18
B-05	16+00	3	N/A	N/A	1.5	18
B-06	18+00	2.5	N/A	N/A	1.5	18
B-07	20+00	2	N/A	N/A	2	12
B-08	22+00	3	N/A	N/A	2	12
B-09	24+00	3	N/A	N/A	3	12
B-10	26+00	3.5	N/A	N/A	2.5	12
B-11	28+00	2.5	N/A	N/A	2	12
B-12	28+00*	3.75	N/A	N/A	N/A	18
B-13	30+00	4	N/A	N/A	2	12
B-14	32+00	3	N/A	N/A	2	11
B-15	34+00	3	N/A	N/A	1.5	18
B-16	36+00	3	N/A	N/A	2.5	18
B-17	38+00	6	N/A	18	N/A	N/A
B-18	40+00	8	N/A	8	N/A	N/A
B-19	42+00	8	N/A	6	N/A	N/A
B-20	42+00*	7	N/A	4	N/A	N/A
B-21	45+00	6	N/A	6.5	N/A	N/A
B-22	47+00	6	N/A	4	N/A	N/A
B-23	49+00	5	N/A	5	N/A	N/A

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			Existing Pa	vement Thick	ness (inches)											
Boring No.	Station No.	Asphalt Concrete		Asphalt Treated Aggregate Base Course	Aggregate Base Course	Select Fill/ Sand and Gravel Subbase										
B-24	51+00	4.5	N/A	5	N/A	N/A										
B-25	53+00	4	N/A	2.5	N/A	N/A										
B-26	55+00	6	4	6	N/A	N/A										
B-27	57+00	8	N/A	4	N/A	N/A										
B-28	59+00	7	N/A 8 N		N/A 8 N/A		N/A 8 N/A		N/A 8 N/A		N/A 8 N/A		N/A 8		N/A	N/A
B-29	B-29 59+00*		N/A	8	N/A	N/A										

<sup>\*</sup>Cross taxiway

# 4.2 Subgrade and Subsurface Soil Conditions

Specific conditions encountered at the boring locations are indicated on the individual boring logs. Stratification boundaries on the boring logs represent the approximate location of changes in soil types; in-situ, the transition between materials may be gradual. Details for the borings can be found on the attached boring logs. Based on the results of the borings, subsurface conditions on the project site can be generalized as follows:

Description	Approximate Depth to Bottom of Stratum (feet)	Consistency/Density	
		Asphalt – 2 to 7.5 inches	
		Concrete* – 4 inches	
Otrostoros 4	0.9 to 1.5	Asphalt Treated Base Course** –	N/A
Stratum 1	0.9 (0 1.5	1.5 to 2.5	IN/A
		Aggregate Base Course***	
		Select Fill**** – Sand and Gravel	
Ctuation 0	4 to 11.5	Lean to Fat Clay. The sand and	Soft to Very Stiff
Stratum 2	4 10 11.5	gravel content varied	Soft to very Still
01:-1 0	13	Silt. The sand and gravel content	Hard
Stratum 3	13	varied	Паги
01:-1 1	2 to 15	Gravel. The clay, silt, sand and	Very Leage to Dance
Stratum 4	2 10 15	cobble content varied	Very Loose to Dense
Stratum 5	6 to 15	Sand. The silt, clay and gravel	Loose to Very Dense
_		content varied	_

<sup>\*</sup>Encountered in Boring No. B-26

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Laboratory tests were conducted on selected soil samples and the test results are presented on the attached laboratory test sheets in Appendix B.

The four (4) bulk samples that were retrieved during the field exploration and taken to the laboratory for further observation by the project geotechnical engineer were classified in accordance with the Unified Soil Classification System (USCS), the American Association of State Highway and Transportation Officials (AASHTO) classification system, and by the FAA Frost Group (AC 150/5320-6E) as outlined below.

Boring No.	USCS Classification	AASHTO Classification	FAA Frost Group
B-02	SM	A-2-6	FG-3
B-13	GC	A-2-6	FG-3
B-21	CL	A-7-6	FG-4
B-29	СН	A-7-6	FG-4

Four (4) Atterberg limits tests were performed on the bulk samples of the subgrade obtained from depths ranging from 1 to 5 feet below existing site grades. Liquid Limit (LL) values for the subgrade materials ranged from 38 to 56. The plasticity index (PI) values for the subgrade materials ranged from 13 to 32. The percent fines of the bulk samples ranged from 32 to 69%.

The in-situ dry densities and moisture contents of the subgrade soils ranged from about 71 to 115 pounds per cubic foot (pcf) and 2.5 to 32 percent, respectively.

Laboratory test results indicate that the surface and near surface soils typically exhibit low consolidation/compression potential at in-situ moisture contents. The near surface soils exhibit a low to high tendency for consolidation/compression and non- to low tendency for expansion (swell) when wetted under heavy pavement loads. When water is added to samples of laboratory compacted near-surface soils, we anticipate that the compacted soils will exhibit non- to low expansive potential and non- to low consolidation potential when subjected to light loading conditions such as those imposed by pavements.

Four (4) moisture density relations (Proctor) tests were performed on selected samples of the subgrade obtained from depths ranging from 1 to 5 feet below existing site grades. The results of this testing is summarized below:

<sup>\*\*</sup> Encountered in Boring Nos. B-17 through B-29

<sup>\*\*\*</sup>Encountered in Boring Nos. B-01 through B-11 and B-13 through B-16

<sup>\*\*\*\*</sup>Encountered in Boring Nos. B-01 through B-16

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Boring No.	USCS Classification	AASHTO Classification	Maximum Dry Density (pcf)	Optimum Moisture Content (%)
B-02	SM	A-2-6	108.5	16.6
B-13	GC	A-2-6	112.3	15.1
B-21	CL	A-7-6	102.4	20.0
B-29	СН	A-7-6	96.0	24.9

Four (4) California Bearing Ratio (CBR) tests were performed on selected samples of the subgrade obtained from depths ranging from 1 to 5 feet below existing site grades. The CBR tests were performed on subgrade samples compacted at approximately 95% maximum dry density and at or near optimum moisture content determined by ASTM D1557. A standard surcharge load of 10 pounds and saturated conditions were used for the tests. CBR values for the subgrade materials are summarized below:

Boring No.	USCS Classification	AASHTO Classification	CBR Value (0.100")	CBR Value (0.200")
B-02	SM	A-2-6	14.7	8.6
B-13	GC	A-2-6	9.9	7.4
B-21	CL	A-7-6	5.1	3.8
B-29	СН	A-7-6	8.0	6.2

The in-situ moisture contents of the subgrade soils ranged from 3.9 to 20.5%, the in-situ densities of the soils ranged from 89 to 111 pounds per cubic foot (pcf). Detailed in-situ moisture content and dry density values are shown on the individual boring logs and in the Summary of Laboratory Results Table in Appendix B.

The results of the soluble sulfate testing on the four (4) bulk samples are summarized below:

Boring No.	USCS Classification	AASHTO Classification	Soluble Sulfates (mg/kg)
B-02	SM	A-2-6	3,100
B-13	GC	A-2-6	100
B-21	CL	A-7-6	170

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Boring No.	USCS	AASHTO	Soluble Sulfates
	Classification	Classification	(mg/kg)
B-29	СН	A-7-6	3,300

# 5.0 GROUNDWATER INFORMATION

Groundwater was not observed in the test borings at the time of field exploration, nor when checked upon completion of drilling. These observations represent groundwater conditions at the time of the field exploration and may not be indicative of other times, or at other locations. Groundwater conditions can change with varying seasonal and weather conditions, and other factors.

Fluctuations in groundwater levels can best be determined by implementation of a groundwater monitoring plan. Such a plan would include installation of groundwater monitoring wells, and periodic measurement of groundwater levels over a sufficient period of time.

The possibility of groundwater fluctuations should be considered when developing design and construction plans for the project.

# 6.0 ANALYSIS AND RECOMMENDATIONS

#### 6.1 Geotechnical Considerations

The site appears suitable for the proposed pavement reconstruction based upon existing pavement and geotechnical conditions encountered in the test borings. The on-site subsurface soils are considered to be relatively poor to fair quality subgrade materials for aircraft support.

Variable subsurface subgrade conditions were encountered that will require particular attention in the design and construction; soft to very stiff clays and loose to very dense sands. In addition, elevated moisture contents and potentially unstable soils were encountered in the majority of the borings. Therefore, some stabilization or drying of the subgrade materials should be anticipated and required during construction, especially during wet seasons. Potential stabilization measures are outlined in subsequent sections of this report.

The observed pavement distress included transverse and longitudinal cracking, as well as oxidation, weathering and raveling of the pavement surface.

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The existing pavement section thickness was variable in terms of the type and thickness of materials across the taxiway. It appears that the pavement section for approximate western ½ of the taxiway incorporated asphalt concrete and aggregate base course overlying a zone of select fill. It appears that the pavement section for approximate eastern ½ of the taxiway incorporated asphalt concrete and asphalt treated base course. The pavement materials consisted of asphalt concrete, Portland cement concrete, asphalt treated base course, aggregate base course and select fill. The asphalt pavement surface course varied from 2 to 8 inches. The Portland cement concrete, aggregate base course, asphalt treated base course and/or select fill were encountered in the borings. Four (4) inches of Portland cement concrete was encountered beneath the asphalt concrete in one (1) boring. The aggregate base course ranged from 1.5 to 3 inches. The asphalt treated base course ranged from 4 to 18 inches. The select fill ranged from 12 to 18 inches.

Due to the variability in the pavement materials type and thickness and the requirement to maintain existing site grades, it is our opinion that in-situ reclamation or recycling of the existing pavement materials is not feasible.

# 6.2 Corrosivity

Laboratory test results indicate the on-site soils exhibit soluble sulfate concentrations of 100 and 3,300 mg/kg. Results of soluble sulfate testing indicate that ASTM Type V or a modified Type II Portland cement should be considered for all project concrete on and below grade. Foundation concrete should be designed for moderate sulfate exposure in accordance with the provisions of the ACI Design Manual, Section 318, Chapter 4.

# 7.0 EARTHWORK

# 7.1 Site Preparation

Strip and remove existing pavement materials, and other deleterious materials from proposed new pavement reconstruction areas. Exposed surfaces should be free of mounds and depressions which could prevent uniform compaction.

Evidence of underground facilities and utilities were observed within and near the project site during the site reconnaissance. Therefore, such features could be encountered during construction. If unexpected fills or underground facilities are encountered, such features should be removed and the excavation thoroughly cleaned prior to backfill placement and/or construction.

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# 7.2 Excavation

It is anticipated that shallow excavations for the proposed construction can be accomplished with conventional earthmoving equipment.

Based upon the borings, we do not anticipate groundwater will be encountered in excavations up to 15 feet in depth. Pumping from sumps may be utilized to control water within excavations.

Based upon the subsurface conditions determined from the geotechnical exploration, subgrade soils exposed during construction will likely encounter unstable conditions. Soft and loose subgrade conditions along with elevated moisture contents were encountered in the majority of the borings. Moisture contents as high as 32 percent were encountered in the borings. There, unstable subgrade conditions should be anticipated and expected during construction. The stability of the subgrade may be affected by precipitation, repetitive construction traffic or other factors. If unstable conditions develop, workability may be improved by scarifying and drying. Overexcavation of wet zones and replacement with granular materials may be necessary. Use of lime, fly ash, kiln dust, cement or geotextiles/geogrids could also be considered as a stabilization technique. Laboratory evaluation is recommended to determine the effect of chemical stabilization on subgrade soils prior to construction. Lightweight excavation equipment may be required to reduce subgrade pumping.

The individual contractor(s) is responsible for designing and constructing stable, temporary excavations as required to maintain stability of both the excavation sides and bottom. Excavations should be sloped or shored in the interest of safety following local, and federal regulations, including current OSHA excavation and trench safety standards.

# 7.3 Pavement Maintenance and Surface Drainage

Preventative maintenance consists of both localized maintenance (e.g. crack sealing and patching) and global maintenance (e.g. surface sealing). Preventative maintenance is usually the first priority when implementing a planned pavement maintenance program and provides the highest return on investment for pavements.

Pavement design methods are intended to provide structural sections with adequate thickness over a particular subgrade such that wheel loads are reduced to a level that subgrade can support. The support characteristics of the subgrade for pavement design do not account for the shrink movements of the soils encountered on this project. Thus the pavement may be adequate for a structural standpoint, yet still experience cracking and deformation due to shrink related movement of the subgrade. It is, therefore, important to minimize moisture changes in the subgrade to reduce collapse/consolidation movements.

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Future performance of pavements constructed at the airport facility will be dependent upon several factors, including:

- Maintaining stable moisture content of the subgrade soils.
- Providing for a planned program of preventative maintenance.

The performance of all pavements can be enhanced by minimizing excess moisture which can reach the subgrade soils. The following recommendations could be considered:

- site grading at a minimum 2% grade away from the pavements;
- subgrade and the pavement surface have a minimum ¼ inch per foot slope to promote proper surface drainage.
- consider appropriate edge drainage and pavement under drain systems,
- install pavement drainage surrounding areas anticipated for frequent wetting
- install joint sealant and seal cracks immediately,
- compaction of any utility trenches for landscaped area to the same criteria as the pavement subgrade.
- seal all landscaped areas in, or adjacent to pavements to minimize or prevent moisture migration to subgrade soils;

# 8.0 GENERAL COMMENTS

Terracon should be retained to provide observation and testing services during grading, excavation, pavement construction and other earth-related construction phases of the project.

The analysis and recommendations presented in this report are based upon the data obtained from the borings performed at the indicated locations and from other information discussed in this report. This report does not reflect variations that may occur between borings, across the site, or due to the modifying effects of construction or weather. The nature and extent of such variations may not become evident until during or after construction. If variations appear, we should be immediately notified so that further evaluation and supplemental recommendations can be provided.

The scope of services for this project does not include either specifically or by implication any environmental or biological (e.g., mold, fungi, bacteria) assessment of the site or identification or prevention of pollutants, hazardous materials or conditions. If the owner is concerned about the potential for such contamination or pollution, other studies should be undertaken.

This report has been prepared for the exclusive use of our client for specific application to the project discussed and has been prepared in accordance with generally accepted geotechnical engineering practices. No warranties, express or implied, are intended or made. Site safety,

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excavation support, and dewatering requirements are the responsibility of others. In the event that changes in the nature, design, or location of the project as outlined in this report are planned, the conclusions and recommendations contained in this report shall not be considered valid unless Terracon reviews the changes and either verifies or modifies the conclusions of this report in writing.

We have appreciated being of service to you in the geotechnical field and laboratory testing phase of this project. If you have any questions concerning this letter, test results, and consulting services, please do not hesitate to contact us.

Principa

Sincerely,

Terracon Consultants, Inc.

Sam G. Zsiga

Staff Engineer

Enclosures: Appendix A

Site Location Map

**Exploration Location Plan** 

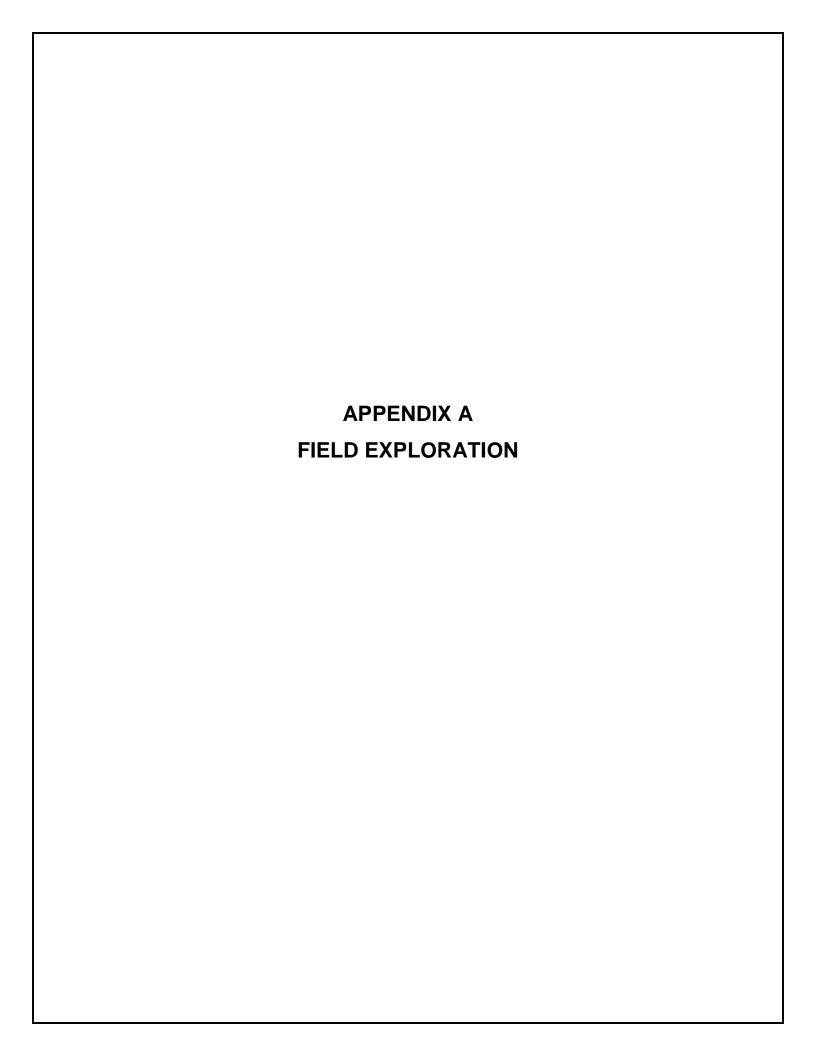
Boring Logs General Notes

Unified Soil Classification System

Appendix B

Laboratory Test Results

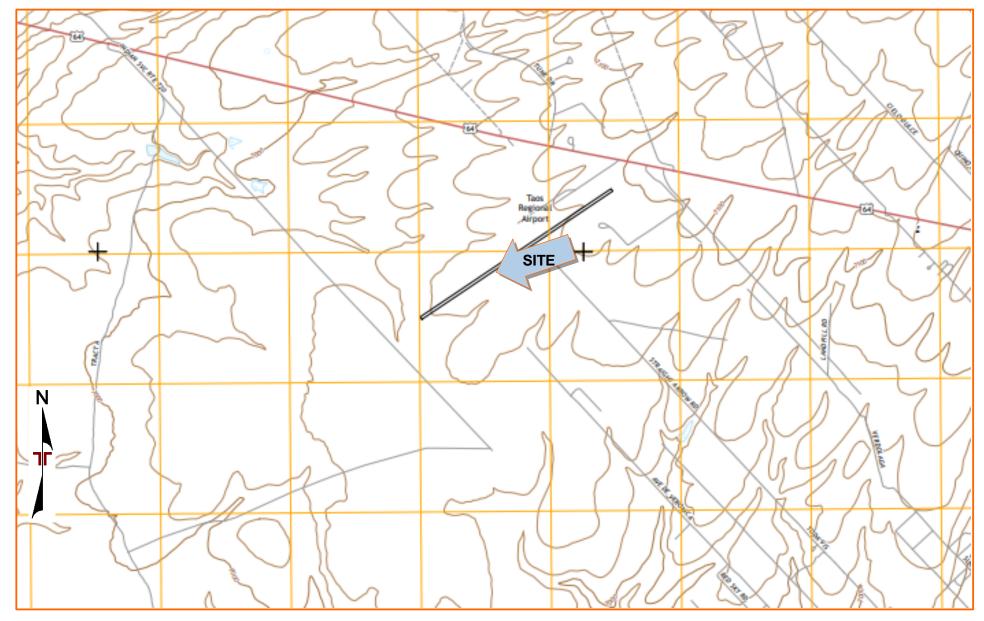
Responsive Resourceful Reliable



#### SITE LOCATION

Taos Regional Airport Taos, NM
June 8, 2018 Terracon Project No. 66185006





#### **EXPLORATION PLAN**

Taos Regional Airport ■ Taos, NM

June 8, 2018 Terracon Project No. 66185006





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9	LOCATION See Exhibit A-2			NS II	)E			STF	RENGTH	TEST	(9	٦	ATTERBERG LIMITS	ES
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	1.8 SELECT FILL - 18"													
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보 2	13.0 SILTY SAND (SM), brown		-	-										
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		See Exhibit A-3 for procedures.	descri	ption of	field		Notes:							
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ם. היים היים		5.0		5 -	4										
Ę .		SILTY SAND (SM), white, very dense				X	11-23					22			32
EMF		<mark>(</mark>				$\triangle$	N=	56							
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ב בי				_											
2		<mark>(</mark>													
TAT .				-											
2		<u> </u>		10-	-										
5		<u> 8</u>		_		X	26-	50				16			
A .		<mark>:</mark>													
		<mark>.</mark>		-											
A H				-											
ACS															
90				-											
0681 1830	ŀ	15.0  Boring Terminated at 15 Feet		15-	-										
8 _		Borning reminiated at 101 eet													
WEL															
Q N															
GEO SMAKI LOG															
- ¥															
N N															
S E															
l															
GINAL REPORT															
7 7															
Y N															
<u> </u>															
<u>S</u>															
7															
\$		Stratification lines are approximate. In-situ, the transition ma	y be gradual.					Hamme	r Typ	e: Autom	atic	•		•	•
H															
		incement Method: ollow Stem Auger	See Exhibit A-3 for oprocedures.	descri	ption of	field		Notes:							
ALID			See Appendix B for procedures and add	descr	iption o	f labo	ratory								
- Δh	anr	ndonment Method:	procedures and add See Appendix C for												
Z   F	Bor	orings backfilled with soil cuttings upon completion. saled with bituminous cold patch at surface.	abbreviations.	.,,,,,,		- ,									
∄L`	sea		L <u> </u>												
- NG		WATER LEVEL OBSERVATIONS	75					Boring Sta	arted:	05-03-20	18	Borir	ng Com	pleted: 05-03-	2018
Ž -			ller	_				Drill Rig: (	CME 7	75		Drille	er: EDI		
					ns St N ue. NM			Proiect No	o.: 661	185006		Exhi	oit:	A-7	

	В	ORING I	LO	G N	10	. B-0	7					F	Page 1 of	1
PR	OJECT: Taos Regional Airport			CLIE	NT:	Arms Albud	trong C querque	on: e, N	sultan M	ıts In	С			
SIT	E: Taos Regional Airport Taos, NM													
GRAPHIC LOG	LOCATION See Exhibit A-2 Latitude: 36.457° Longitude: -105.6762°		DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST	SULTS	TEST TYPE S	COMPRESSIVE STRENGTH D D H	STRAIN (%)	WATER CONTENT (%)	DRY UNIT WEIGHT (pdf)	ATTERBERG LIMITS	PERCENT FINES
GRA	Station: 20+00  DEPTH  0.2 AASPHALT - 2"		DEI	WAT	SAM		<u> </u>	TEST	COMPR STRE (ts	STRA	SOO	WEI		PERC
	0.3. AGGREGATE BASE COURSE - 2" SELECT FILL - 12"			-										
	SANDY LEAN CLAY WITH GRAVEL (CL), brow	wn, stiff			X	1	2				21	97		70
			_	-	X	6-7 N=					25			67
			5 -	_	X	3-4 N=					25			57
				_										
	9.0  CLAYEY GRAVEL (GC), brown, dense		10-											
				_	X	2-15 N=					8			
	15.0  Boring Terminated at 15 Feet		15 <sup>-</sup>											
	Stratification lines are approximate. In-situ, the transition may	be gradual.					Hamme	rTyp	e: Autom	natic				
	ow Stem Auger	See Exhibit A-3 for oprocedures. See Appendix B for oprocedures and add	desc	ription o al data (i	of labo		Notes:							
Bori	ngs backfilled with soil cuttings upon completion. led with bituminous cold patch at surface.	See Appendix C for abbreviations.	expla	anation o	ot syn	nbols and								
	WATER LEVEL OBSERVATIONS	Ter		7			Boring Sta			)18			pleted: 05-03-	2018
		4905	Hawk	ins St N	IE	71 1	Drill Rig: 0				-	er: EDI		
				que, NM			Project No	o.: 66	185006		Exhi	bit:	A-9	

	В	ORING I	LO	G N	10	. B-0	9					F	Page 1 of	1		
PF	OJECT: Taos Regional Airport			CLIE	NT:	Arms	trong C querque	Con: e, N	sultan M	ts In	С		<u> </u>			
SI	ΓΕ: Taos Regional Airport Taos, ΝΜ															
Ö	LOCATION See Exhibit A-2			LS SS	ñ			STF	RENGTH	TEST	<u></u>		ATTERBERG LIMITS	SII		
GRAPHIC LOG			DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST	ST.	Ш	COMPRESSIVE STRENGTH (tsf)	<u> </u>	WATER CONTENT (%)	DRY UNIT WEIGHT (pd)		PERCENT FINES		
₹	Latitude: 36.4576° Longitude: -105.6751°		Ĭ	Z.¥	벨	D TI	Jns	TEST TYPE	SSI IGT	STRAIN (%)	A L	탏		F		
₹	Station: 24+00		닖	SHA SHR	MP	ΞË	RE	STI	RA Fig.	RAII	ÌŠĘ	EG PR	LL-PL-PI	22		
Ŋ	DEPTH			Š.	SA	ш		TE	NO.	ST	ŏ	>		H H		
	0.3 ASPHALT - 3"															
2	0.5 AGGREGATE BASE COURSE - 3"	/	_													
	SELECT FILL - 12"															
	SILTY GRAVEL (GM), trace clay, light brown,	medium	-	-	$ \nabla $	8-8					12			12		
200	<sub>3.0</sub> dense				$/\backslash$	N=	:14				'-			'-		
	LEAN TO FAT CLAY (CL/CH), brown to light be medium stiff to stiff	rown,														
	medium stiff to stiff		-			1	3				23	83		72		
			_													
			5 -		$\bigvee$	2-3	3-6									
			-	_	M	N=					28			87		
					$\longrightarrow$											
			_													
			-													
			-													
OLD LEWSCON PAIN			10-	4												
					$ \mathcal{V} $	2-2					22					
	11.5		-		$\triangle$	N=	=5									
	SILTY GRAVEL (GM), light brown		-	_												
			-													
			_													
	15.0															
	Boring Terminated at 15 Feet		15-													
4																
2																
b b																
Ď																
=																
į																
Į																
5																
5																
2																
\$	Stratification lines are approximate. In-situ, the transition may	y be gradual.		•			Hamme	r Typ	e: Autom	atic			•	•		
Ž																
		See Exhibit A-3 for	descri	iption of	field		Notes:									
HO		procedures. See Appendix B for	desar	intion o	f laha	rator.										
\$		procedures and add														
		See Appendix C for	expla	nation	of syn	nbols and										
Bo Se	ings backfilled with soil cuttings upon completion.  aled with bituminous cold patch at surface.	abbreviations.														
$\vdash$	WATER LEVEL OBSERVATIONS						<del>                                     </del>				L					
									Boring Completed: 05-03-2018							
				ins St N			Drill Rig: (	CME	75		Drille	er: EDI				
<b>[</b> ]				ue, NM			Project No	o.: 66	185006		Exhi	bit:	A-11			

	E	BORING L	0	G N	10	. B-1	1					F	Page 1 of	1
PF	ROJECT: Taos Regional Airport		1	CLIE	NT:	Armst	trong C	ons e, N	sultan M	ts In	С		<u> </u>	
SI	TE: Taos Regional Airport Taos, NM													
GRAPHIC LOG	LOCATION See Exhibit A-2 Latitude: 36.4582° Longitude: -105.6737° Station: 28+00  DEPTH  0.2 ASPHALT - 2.5"  0.4 AGGREGATE BASE COURSE - 2"  SELECT FILL - 12"  SILTY GRAVEL (GM), with clay, light brown, I		DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	8-4 N=	l-4	TEST TYPE	COMPRESSIVE Z STRENGTH D (tsf)	STRAIN (%)	T WATER CONTENT (%)	DRY UNIT WEIGHT (pcf)	ATTERBERG LIMITS	PERCENT FINES
	3.0 SANDY FAT CLAY (CH), brown to tan, stiff		- 5 - -		X	2-3 N=	3-7				14	98		60
	9.0  SILTY GRAVEL (GM), light brown to medium dense	1	- - - 10 - - -			21-3 <sup>,</sup> N=	4-37				3			
	Boring Terminated at 15 Feet  Stratification lines are approximate. In situ, the transition res		15-				Hamma	- T.	Autom	atia				
	Stratification lines are approximate. In-situ, the transition ma	y be gradual.						r Iype	e: Autom	atic				
Ho Aban Bo	cement Method: llow Stem Auger  donment Method: rings backfilled with soil cuttings upon completion. aled with bituminous cold patch at surface.	See Exhibit A-3 for de procedures. See Appendix B for de procedures and additi See Appendix C for ex abbreviations.	escri onal	iption o I data (i	f labo f any)		Notes:							
	WATER LEVEL OBSERVATIONS	760	stacon						05-03-20	18	Borir	ng Com	oleted: 05-03-	2018
		4905 Ha	awkii	ns St N	E		Drill Rig: C				+	er: EDI	. 42	
- 1		Albugu	Jera	ue. NM			Project No	661	185006		L⊨xhi	bit: /	<del>\</del> -13	

	E	BORING	LO	G N	10	. B-1	3					F	Page 1 of	1	
PR	OJECT: Taos Regional Airport			CLIE	NT:	Armst Albuq	trong C	ons e, N	sultan M	ts In	С				
SI	ΓΕ: Taos Regional Airport Taos, ΝΜ														
GRAPHIC LOG	LOCATION See Exhibit A-2 Latitude: 36.4585° Longitude: -105.6734° Station: 30+00		DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST	RESULTS	TEST TYPE S	COMPRESSIVE STRENGTH D D (tsf) T	STRAIN (%)	WATER CONTENT (%)	DRY UNIT WEIGHT (pcf)	ATTERBERG LIMITS LL-PL-PI	PERCENT FINES	
	DEPTH			-0	o			_	8"	0)			00.47.04		
	AGGREGATE BASE COURSE - 2"		-	_	M	13-13	3-13				8		<del>38-17-21</del>	23	
	1.5 2.0 CLAYEY GRAVEL WITH SAND (GC), brown, dense LEAN TO FAT CLAY (CL/CH), tan, very stiff	medium	-			N=								20	
			-	4	X	2	1				31	84		76	
			5 -												
			-		X	3-11 N=:	-16 27				26			78	
	9.0 SILTY GRAVEL (CH), trace cobbles, light bro	own, dense	10-												
					X	9-19 N=					5			13	
	15.0		- - 15-												
	Boring Terminated at 15 Feet														
	Stratification lines are approximate. In-situ, the transition ma	av be gradual.					Hamme	r Type	e: Autom	natic					
								76.							
Hol	Icement Method: Ilow Stem Auger Ionment Method: Ings backfilled with soil cuttings upon completion. Illed with bituminous cold patch at surface.	See Exhibit A-3 for procedures. See Appendix B for procedures and add See Appendix C for abbreviations.	descr	ription o	f labo f any)		Notes:								
	WATER LEVEL OBSERVATIONS	75-	lerracon						05-03-20	Borir	Boring Completed: 05-03-2018				
					_		Drill Rig: C	CME 7	75		Drille	er: EDI			
				ins St N			Project No.: 66185006 Exhibit: A-15								

	BORING L	_0	G I	10	. B-1	7					F	Page 1 of	1
PROJECT: Taos Regional Airport			CLIE	NT:	Arms	trong Co	ons	sultan M	ts In	С		<u> </u>	
SITE: Taos Regional Airport Taos, NM							,						
LOCATION See Exhibit A-2  Latitude: 36.4597° Longitude: -105.6711°  Station: 38+00		DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST	တ္ 🕇	TEST TYPE S	COMPRESSIVE STRENGTH D COMPRESSIVE (tsf)	STRAIN (%) LS	WATER CONTENT (%)	DRY UNIT WEIGHT (pdf)	ATTERBERG LIMITS LL-PL-PI	PERCENT FINES
DEPTH 0.5 ASPHALT - 6" ASPHALT TREATED BASE COARSE - 1	18"	-	-0	0)				300	0,				
2.0 FAT CLAY (CH), trace gravel, red brown	ı, loose			X	1	3				10	115		22
4.0 SANDY LEAN CLAY (CL), light brown, s	tiff	5 -		X	2-4 N=					22			68
				X	3-4 N=					25			66
8.0 SILTY GRAVEL (GM), brown, very dens	e												
		10-		X	11-3 N=					3			
		-	_										
Boring Terminated at 15 Feet		15-											
Stratification lines are approximate. In-situ, the transit	ion may be gradual.					Hammer	Туре	e: Autom	atic				
Advancement Method: Hollow Stem Auger	See Exhibit A-3 for oprocedures. See Appendix B for procedures and additional additional and additional additi	desci itiona	ription o Il data (	f labo if any)		Notes:							
Abandonment Method: Borings backfilled with soil cuttings upon completion.	See Appendix C for abbreviations.	expla	ination	ot sym	nbols and								
WATER LEVEL OBSERVATIONS	= 1Teri	۲	<b>)</b> [			Boring Star			18		ng Comp	oleted: 04-27-2	2018
	4905 Hawkins St NE Albuquerque, NM Project No.: 66185006 Exhibit:								\-19				

		BORING LO						```	nultar.	to In		F	Page 1 of	1	
SIT			CL	.IET	NI:	Albud	trong C querque	e, N	suitan M	ts in	С				
(2)	Taos, NM  LOCATION See Exhibit A-2		Ι,	<u>S</u>	ш			STF	RENGTH	TEST			ATTERBERG LIMITS	ပ္ပ	
GRAPHIC LOG	Latitude: 36.4601° Longitude: -105.6705° Station: 40+00	DEPTH (Ft.)	WATER LEVEL	OBSERVATION	SAMPLE TYPE	FIELD TEST	RESULTS	TEST TYPE	COMPRESSIVE STRENGTH (tsf)	STRAIN (%)	WATER CONTENT (%)	DRY UNIT WEIGHT (pdf)	LL-PL-PI	PERCENT FINES	
	DEPTH 0.7 ASPHALT - 8"								0						
	1.3 ASPHALT TREATED BASE COARSE - 8"  SANDY FAT CLAY WITH GRAVEL (CH), dark stiff	s brown,			X	6-5 N=					18			61	
	CLAYEY SAND (SC), trace to with gravel, brobrown, loose	own to dark				1	8				9	105		26	
		5			X	15- N=					11			36	
<u> </u>	Boring Terminated at 6.5 Feet			_/		14-									
	Stratification lines are approximate. In-situ, the transition ma	ay be gradual.					Hamme	ег Тур	e: Autom	atic					
Holl band	cement Method: ow Stem Auger  onment Method: ngs backfilled with soil cuttings upon completion.	See Exhibit A-3 for desc procedures. See Appendix B for desc procedures and addition See Appendix C for expl abbreviations.	cription	n of a (if	laboi any)		Notes:								
	WATER LEVEL OBSERVATIONS	Terracon					Boring Started: 04-27-2018					Boring Completed: 04-27-2018			
		4905 Haw		_			Drill Rig: (				Drille	er: EDI			
		Albuquer		Project No.: 66185006					Exhibit: A-20						

	Е	BORING	LO	G I	O	. B-1	9					F	Page 1 of	1	
F	ROJECT: Taos Regional Airport			CLIE	NT:	Arms	trong C querque	ons	sultan м	ts In	С				
5	SITE: Taos Regional Airport Taos, NM					Albuq	querque	, IV							
۳	LOCATION See Exhibit A-2		_	H <sub>S</sub> N	)든	_		STF	RENGTH	TEST	(%)	f)	ATTERBERG LIMITS	ES	
GRAPHICLOG	Latitude: 36.4604° Longitude: -105.67°		DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST	LTS	밁	COMPRESSIVE STRENGTH (tsf)	(%	WATER CONTENT (%)	DRY UNIT WEIGHT (pd)		PERCENT FINES	
APH	Station: 42+00		Ë	TER.	1PLE		ESU	TEST TYPE	RES ENG	STRAIN (%)	WAT	7 E	LL-PL-PI	<u>N</u>	
S. S.			ä	WA	SAN	≣	Ľ	TES	STR	STR	8	<sup>™</sup>		PER	
	DEPTH 0.7 ASPHALT - 8"								0						
0	1.2 ASPHALT TREATED BASE COARSE - 6"			_											
	CLAYEY SAND WITH GRAVEL (SC), light bromedium dense	wn,			X	20-1 N=					11			28	
	mediam dense				$\vdash$	14-	17								
<u> </u>	3.5														
	LEAN CLAY/FAT CLAY (CL/CH), brown, stiff	to very stiff		-		6-7					20			51	
			5 -			N=	:15								
5					X	1	5				24	100		75	
<u> </u>															
	8.0 SILTY GRAVEL (GM), light brown, medium de	ense		_											
3	Sierr Stavee (Sim), light brown, modium at	51100													
			10												
F 6			10-		$\bigvee$	8-1	1-9				9				
						N=	:20				9				
	∳ 12.0 SILTY SAND (SM), trace gravel, brown			_											
S L	: :: ::														
PS .															
900	15.0														
CO Q	Boring Terminated at 15 Feet		15-												
4															
2															
LOG-18															
2															
N N															
GEO SIMARI															
-															
אר ה															
7 7															
5															
5															
2															
	Stratification lines are approximate. In-situ, the transition ma	y be gradual.					Hamme	r Type	e: Autom	l natic					
7 4 2															
	ancement Method: lollow Stem Auger	See Exhibit A-3 for	r descr	iption o	f field		Notes:								
}	····g	procedures. See Appendix B fo	r desc	ription o	of labo	ratory									
Δh	indonment Method:	procedures and ad See Appendix C fo	lditiona	ıl data (	if any)	).									
	forings backfilled with soil cuttings upon completion.	abbreviations.			,,,,										
	WATER LEVEL OBSERVATIONS						<u> </u>				T_			00:5	
				7			Boring Sta			)18	_		oleted: 04-27-	2018	
				ins St N	_		Drill Rig: 0	CME 7	/5		Drille	er: EDI			
Ē				ue. NV			Project No	o.: 661	185006		Exhi	bit: A	\-21		

		BORING	LO					<b>.</b>		4- '	_	F	Page 1 of	1
PR	OJECT: Taos Regional Airport  E: Taos Regional Airport			CLIE	:N Γ:	Arms	trong ( querque	cons e, N	sultan M	ts In	С			
	Taos, NM			T									ATTERBERG	
GRAPHIC LOG	LOCATION See Exhibit A-2 Latitude: 36.4605° Longitude: -105.6693° Station: Cross Taxiway Station No. 42+00		DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	LD TEST	RESULTS	TEST TYPE G	COMPRESSIVE STRENGTH LT (tsf)	STRAIN (%)	WATER CONTENT (%)	DRY UNIT WEIGHT (pd)	LIMITS  LL-PL-PI	PERCENT FINES
GRA	DEPTH 0.6 ASPHALT - 7"		DE	WAT	SAMI	믬		TEST	COMPF STRE	STRA	> OO	WE	LL-1 L-1 1	PERC
	0.9 ASPHALT TREATED BASE COARSE - 4"													
	LEAN CLAY (CL), trace gravel, brown, mediustiff	ım stiff to			X		3-5 =8				20			72
					X	1	1				18	98		71
	SANDY LEAN CLAY (CL), trace gravel, brown stiff to stiff	n, medium	5 -		X		4-4 =8				20			66
<u>////</u>	Boring Terminated at 6.5 Feet													
	Stratification lines are approximate. In-situ, the transition ma	ay be gradual.					Hamme	er Type	e: Autom	natic				
duan	cement Method:	1					I Nistani							
Holl	ow Stem Auger	See Exhibit A-3 for procedures. See Appendix B for procedures and ad	r desc ditiona	ription o al data (i	of labo if any)		Notes:							
	onment Method: ngs backfilled with soil cuttings upon completion.	See Appendix C for abbreviations.	ioois and											
	WATER LEVEL OBSERVATIONS	<b>Ter</b>		Boring Started: 04-27-2018 E Drill Rig: CME 75					Boring Completed: 04-27-2018  Driller: EDI					
		4905 Albi		Project No	o.: 66′	185006	Exhi	Exhibit: A-22						

	В	ORING	LO	G I	NO	. B-2	21					F	Page 1 of	1
PF	ROJECT: Taos Regional Airport			CLIE	NT:	Arms	trong C	Cons e, N	sultan M	ts In	С			
SI	TE: Taos Regional Airport Taos, NM													
92	LOCATION See Exhibit A-2			⊩S NS	JE			STF	RENGTH	TEST	(9)	(J	ATTERBERG LIMITS	ES
GRAPHIC LOG	Latitude: 36.4608° Longitude: -105.6691°		DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	TYPE	FIELD TEST	LTS	Ä	COMPRESSIVE STRENGTH (tsf)	(%	WATER CONTENT (%)	DRY UNIT WEIGHT (pd)		PERCENT FINES
APH	Station: 45+00		Ë	HEN.	SAMPLE	::::::::::::::::::::::::::::::::::::	ESU	TEST TYPE	RESS ENG- tsf)	STRAIN (%)	WAT	F 등	LL-PL-PI	EN
GR			ä	WA-	SAN	Ë	<u>~</u>	TES.	STRI	STR,	5			)ER(
	DEPTH 0.5 <b>ASPHALT - 6"</b>			+					Ö					
0 (	1.0 ASPHALT TREATED BASE COARSE - 6.5"			_										
	SANDY LEAN CLAY (CL), trace gravel, brown stiff, some lean clay with sand lenses	medium		_	X	4-2 N=					14 21		49-20-29	69 75
<u> </u>				_										
	FAT CLAY (CH), trace gravel, brown, medium	stiff to stiff		1		3-4 N=					24			83
Į.			5 -	_							40	101		00
				_		1	0				18	104		90
5														
				_										
	9.0 SILTY SAND WITH GRAVEL (SM), light brown	loose		_										
	<u> </u>	,	10-											
					X	3-2					6			
7 0						N=	=5							
				-										
	13.0			_										
	SILTY GRAVEL (GM), light brown													
	15.0													
	Boring Terminated at 15 Feet		15											
5														
<u> </u>														
5														
	Stratification lines are approximate. In-situ, the transition may	be gradual.					Hamme	r Tvo	e: Autom	atic				
2	,,	•						,,,						
		See Exhibit A-3 for	r descr	ription o	f field		Notes:							
		orocedures. See Appendix B fo	r desc	ription o	of labo	ratory								
<u> </u>		orocedures and ad See Appendix C fo	lditiona	al data (	if any)									
		abbreviations.	" GYNIG	ariauUII	oi syiT	inois allu								
	WATER LEVEL ORSERVATIONS													
	WATER LEVEL OBSERVATIONS						Boring Sta	arted:	04-27-20	18	Borir	ng Com	pleted: 04-27-	2018
5							Drill Rig: (	CME	75		Drille	er: EDI		
É				ins St N aue. NN			Project No	o.: 66°	185006		Exhi	bit:	A-23	

	В	ORING	LO	G N	10	. B-2	22					F	Page 1 of 1	1
PF	OJECT: Taos Regional Airport			CLIE	NT:	Arms	trong (	Cons e. N	sultan M	ts In	С			
SI	TE: Taos Regional Airport Taos, NM							-,						
(1)	LOCATION See Exhibit A-2			ļσ	ш			STE	RENGTH	TEST	_		ATTERBERG	Ŋ
GRAPHIC LOG			Ŧ.	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	TSI	ပ		<u>اللـ</u>		WATER CONTENT (%)	DRY UNIT WEIGHT (pdf)	LIMITS	PERCENT FINES
呈	Latitude: 36.4611° Longitude: -105.6686°		DEPTH (Ft.)	ATE	Ē	FIELD TEST		TEST TYPE	COMPRESSIVE STRENGTH (tsf)	STRAIN (%)		S =		
₹	Station: 47+00		딥	E,E	MPL		ZES	T TS	SEN (tsf)	MAS	ŠĘ	[음	LL-PL-PI	E E
D.				N N N	SAI	ш	_	邕	STR	STF	8			H
	DEPTH 0.5 <b>ASPHALT - 6"</b>								0			$\vdash$		
	0.8 ASPHALT TREATED BASE COARSE - 4"													
	CLAYEY GRAVEL (GC), brown, loose													
	,			-	$ \bigvee $	27-	3-3				12			20
	3.0				$/\backslash$	N=	=6							
9	LEAN CLAY/FAT CLAY (CL/CH), trace gravel,	brown to												
	tan, stiff			_	X	1:	5				20	100		70
3.			5 -											
MPLATE.GD1 7/8/			5		М	3-4	1-7				40			
				-	$ \Lambda $	N=	:11				18			59
5														
5				_										
<b>3</b>	9.0													
	SILTY GRAVEL (GM), light brown, medium de	ense												
			10			2.7	7 4							
					X	3-7 N=					3			
	Boring Terminated at 11.5 Feet				$\leftarrow$							$\vdash$		
5	Doring Terminated at 11.57 eet													
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₫	Stratification lines are approximate. In-situ, the transition may	/ be gradual.					Hamme	er Tvo	e: Autom	atic		ш		
2	,,	-					-	71.						
	cement Method:	See Exhibit A-3 for	descr	iption of	field		Notes:							
≐ Ho	low Stem Auger	procedures.												
XY.		See Appendix B for procedures and ad-	r aesc ditiona	πρτιοη ο al data (i	i iabo f any)	ratory ).								
	Ionment Method:	See Appendix C for		,										
Bo	ings backfilled with soil cuttings upon completion.	abbreviations.												
<u> </u>	WATER LEVEL OBSERVATIONS						<b>D</b>		04.0= ==	10	T			0045
		7 Ter					Boring Sta			18	Borir	ng Comp	pleted: 04-27-2	2018
				ins St N			Drill Rig:	CME	75		Drille	er: EDI		
Ĕ				ins St N que, NM			Project No	o.: 66	185006		Exhi	bit: F	<b>A-24</b>	

	В	ORING	LO	G N	10	. B-2	23					F	Page 1 of	1
PR	OJECT: Taos Regional Airport			CLIE	NT:	Arms	trong C	ons	sultan M	ts In	С			
SI	TE: Taos Regional Airport Taos, NM							,						
9	LOCATION See Exhibit A-2			∃.S	)E			STF	RENGTH	TEST	(9)	(	ATTERBERG LIMITS	ES
GRAPHIC LOG	Latitude: 36.4614° Longitude: -105.668°		DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST	57.	Щ	COMPRESSIVE STRENGTH (tsf)	(%)	WATER CONTENT (%)	DRY UNIT WEIGHT (pd)		PERCENT FINES
F	Station: 49+00		PTH	ERI	PLE		lns:	TEST TYPE	RESS INGT	STRAIN (%)	YAT!	종등	LL-PL-PI	H
GR/	Station. 49100		В	WAT	SAM	쁜	2	rest	MPF STRE	STR.	> O	ME		ERC
	DEPTH			-0	0)				8"	0,				<u> </u>
	0.4 <u>ASPHALT - 5"</u> 0.8 1.0 <u>ASPHALT TREATED BASE COARSE - 5"</u>													
	WELL GRADED GRAVEL WITH SAND (GW),	brown	_											
	LEAN TO FAT CLAY (CL/CH), brown, medium		-		X	7	7				26	95		83
			_											
	4.0													
	SANDY LEAN CLAY (CL), trace gravel, tan, m	edium stiff	-		IXI	2-2 N=					21			
<u> </u>	to stiff		5 -	_	$\langle \cdot \rangle$									
					X	3-5 N=					22			57
					$\wedge$									
4 ////			-											
			-	_										
	9.0													
	SILTY GRAVEL (GM), brown, very dense													
			10-	-		44.0	0.50							
Š. P. C.			_		IXI	14-3 N=					7			67
					$\vdash$									
			-											
	13.0 <u>CLAYEY GRAVEL (GC)</u> , brown		-											
	CLATET GRAVEE (GG), BIOWIT		_											
	15.0													
0 2/7/	Boring Terminated at 15 Feet		15-											
1														
<u> </u>														
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	Stratification lines are approximate. In situ the transition most in a	, be gradual					Hamma	r Tim	o: Autom	otio				
<b>{</b>	Stratification lines are approximate. In-situ, the transition may	, be gradual.					namme	тур	e: Autom	iauc				
Advar	ncement Method:	Saa Evhihit 1 2 f	decor	intion o	f fiold		Notes:							
	low Stem Auger	See Exhibit A-3 for procedures.												
\\		See Appendix B for procedures and ad	r descr ditiona	ription c Il data (	of labo if any)	ratory ).								
	Ionment Method:	See Appendix C fo												
Bor	ings backfilled with soil cuttings upon completion.	abbreviations.												
<u> </u>	WATER LEVEL OBSERVATIONS						Doring O		04.07.00	10	D	na C : ::	alatad: 04 07	2040
		ler					Boring Sta			10	+		oleted: 04-27-	.∠U18
			_	ins St N			Drill Rig: (	CME 7	75		Drille	er: EDI		
Ė				ins Sur iue. NM			Project No	o.: 661	185006		Exhi	bit: A	A-25	

	Е	BORING	LO	G N	10	. B-2	25					F	Page 1 of	1
PR	OJECT: Taos Regional Airport			CLIE	NT:	Arms	trong C querque	ons N	sultan M	ts In	С		<u> </u>	
SI	TE: Taos Regional Airport Taos, NM					7 110 00	quoi qui	J, 11						
90	LOCATION See Exhibit A-2			NS E	PE	<b>—</b>		STF	RENGTH	TEST	(%	£	ATTERBERG LIMITS	ES
GRAPHIC LOG	Latitude: 36.462° Longitude: -105.6668°		DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST	JLTS	ЪЕ	COMPRESSIVE STRENGTH (tsf)	(%)	WATER CONTENT (%)	DRY UNIT WEIGHT (pd)		PERCENT FINES
3AP!	Station: 53+00		EPT	ATER SERV	MPL	IELD	RESI	TEST TYPE	PRES RENG (tsf)	STRAIN (%)	WA.	PRY EIG	LL-PL-PI	SCEN
ō	DEPTH			% 8 8 8	SA	ш		Ë	COM	STE	8	_>		H.
	0.3 <u>ASPHALT - 4"</u>	/												
° ()	ASPHALT TREATED BASE COARSE - 2.5" WELL GRADED GRAVEL WITH SAND (GW),	medium /				3:	3				9	103		3
	brown, dense		-											
• <mark>\</mark>	3.0 SANDY LEAN TO EAT CLAY (CL/CH) brown	soft to		_										
	SANDY LEAN TO FAT CLAY (CL/CH), brown, 4.0 medium stiff					1-2	2-2				1.0			
	LEAN CLAY WITH SAND (CL), brown to tan, s medium stiff	soft to	E		M	N=					19			60
4	modium cum		5 -		$\bigvee$	2-3					22			72
					$\triangle$	N=	=8				22			12
			-											
3														
£ ////	9.0		-											
	SILTY GRAVEL (GM), brown to light brown, d	ense	40											
			10-		M	12-2					5			
			-		$\triangle$	N=	:45							
	15.0		15-											
9	Boring Terminated at 15 Feet		15											
4														
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2														
5														
ή Γ														
5														
	Stratification lines are approximate. In-situ, the transition ma	y be gradual.					Hamme	r Type	e: Autom	natic				
2		,												
	cement Method: low Stem Auger	See Exhibit A-3 for procedures.	descri	iption of	field		Notes:							
2	-	See Appendix B for procedures and add	r desci	ription o	f labo	ratory								
	Ionment Method:	See Appendix C for												
Bor	ings backfilled with soil cuttings upon completion.	abbreviations.												
	WATER LEVEL OBSERVATIONS	76					Boring Sta	arted:	04-27-20	)18	Borir	ng Com	oleted: 04-27-	2018
		ller	1	30			Drill Rig: 0					er: EDI		
				ins St N			Project No					bit: A	A-27	

	E	BORING L	.0	G N	10	. B-2	26					F	Page 1 of	1
PF	ROJECT: Taos Regional Airport		(	CLIE	NT:	Arms	trong C	Cons e, N	sultan M	ts In	С			
SI	TE: Taos Regional Airport Taos, NM													
၅	LOCATION See Exhibit A-2			HS S	ř			STF	RENGTH	TEST	(9)		ATTERBERG LIMITS	ES
GRAPHIC LOG	Latitude: 36.4623° Longitude: -105.6664°		DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST	21	m	COMPRESSIVE STRENGTH (tsf)	(9)	WATER CONTENT (%)	DRY UNIT WEIGHT (pd)		PERCENT FINES
PH	Station: 55+00		PTH	ERI	PLE	T 07:	ins:	TEST TYPE	RESS INGT sf)	STRAIN (%)	VATE	종등	LL-PL-PI	H
GR/	Station. 33100		В	WAT	SAM	믬	<u>~</u>	FEST	MPF STRE	STR.	> Ö	WE		ERC
	DEPTH			-0	0)				8"	0)				<u> </u>
90	0.5 <u>ASPHALT - 6"</u> 1.0 <u>ASPHALT TREATED BASE COARSE - 6"</u>													
	CONCRETE - 4"		_											
	LEAN TO FAT CLAY (CL/CH), brown, stiff	/	-			2-4					21			77
	LEAN CLAY WITH SAND (CL), brown, mediu	m stiff to	_		$\triangle$	N=	=9							
	stiff													
5			_		IXI	2-3 N=					21			68
آ ا			5 -	-	$\left\langle \cdot \right\rangle$									
{ ////	6.0		_		$\Delta$	g	9				24	90		74
≦ ⊔ -	Boring Terminated at 6 Feet													
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Į.	Stratification lines are approximate. In-situ, the transition ma	y be gradual.					Hamme	r Type	e: Autom	atic				
Adva	ncement Method:	Con Eveliet A O.S.	loc - ···	nti	: <b>:</b> : - ! - !		Notes:							
	llow Stem Auger	See Exhibit A-3 for d procedures.					1,000.							
\ \ \ \		See Appendix B for opposedures and additional control of the contr	descri tional	iption d I data (i	f labo	ratory ).								
	donment Method:	See Appendix C for 6												
Bo	rings backfilled with soil cuttings upon completion.	abbreviations.												
<u> </u>	WATER LEVEL OBSERVATIONS						Danis - C'		04.00.00	10			-l-4-d-04-02	2042
		Neri					Boring Sta			118			pleted: 04-26-	2018
200		4905 H					Drill Rig: 0	CME 7	75		Drille	er: EDI		
Ē <b>I</b>				ue. NM			Project No	o.: 661	185006		Exhi	bit:	A-28	

	E	ORING I	LO	G N	10	. B-2	27					F	Page 1 of	1
PR	OJECT: Taos Regional Airport			CLIE	NT:	Arms	trong C querque	Cons	sultan M	ts In	С		<u> </u>	
SI	TE: Taos Regional Airport Taos, NM						,	-,						
90	LOCATION See Exhibit A-2		_	SNS	PE	<b>–</b>		STF	RENGTH	TEST	(%	. <del>6</del>	ATTERBERG LIMITS	ES
GRAPHIC LOG	Latitude: 36.4626° Longitude: -105.6658°		DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST	JLTS	Æ	COMPRESSIVE STRENGTH (tsf)	(%)	WATER CONTENT (%)	DRY UNIT WEIGHT (pd)		PERCENT FINES
RAPI	Station: 57+00		ЭЕРТ	ATER SER\	MPL	iELD	RESI	TEST TYPE	PRES RENG (tsf)	STRAIN (%)	WAN	DRY FIG	LL-PL-PI	3CEN
Ö	DEPTH			OB W	SA	ш.		11	COM	ST	ŭ	>		PEF
	0.7 ASPHALT - 8"													
19	CLAVEY SAND WITH CRAVEL (SC) light bro	wn to	-			8	3				25	98		82
	brown, loose		-											
2	LEAN CLAY/FAT CLAY (CL/CH), brown, stiff		-											
					M	3-6					25			79
			5 -			N=	:13				20			13
					X	3-5 N=					24			71
					$\vdash$		11							
			-											
			•	_										
	9.0 <u>SILTY GRAVEL (GM)</u> , trace clay, brown, very	dense	-	-										
			10-	4										
					IXI	17-2 N=					3			
			-											
	15.0  Boring Terminated at 15 Feet		15-											
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Ŕ														
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-	Stratification lines are approximate. In-situ, the transition ma	v be gradual.			Ш		Hamme	r Tvo	e: Autom	natic				
2	,,,,,,	, g						, , ,						
	cement Method: low Stem Auger	See Exhibit A-3 for procedures.	descri	iption of	field		Notes:							
2		See Appendix B for	desci	ription o	f labo	ratory								
Aband	Ionment Method:	procedures and add See Appendix C for												
Bor	ings backfilled with soil cuttings upon completion.	abbreviations.												
	WATER LEVEL OBSERVATIONS	76					Boring Sta	arted:	04-26-20	)18	Borir	ng Com	oleted: 04-26-	2018
		ller		30			Drill Rig: 0					er: EDI		
				ins St N			Project No					bit: A	\-29	

	E	BORING	LO	G N	NO	). B-2	28					F	Page 1 of	1
PF	ROJECT: Taos Regional Airport			CLIE	NT	: Arms	trong C	on:	sultan M	ts In	С			
SI	TE: Taos Regional Airport Taos, NM					Albac	quorqu	J, 14	•••					
9	LOCATION See Exhibit A-2			LI S	Ä			STF	RENGTH	TEST	(9)	(	ATTERBERG LIMITS	ES
GRAPHIC LOG	Latitude: 36.463° Longitude: -105.6652°		DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST	27	Й	COMPRESSIVE STRENGTH (tsf)	(%)	WATER CONTENT (%)	DRY UNIT WEIGHT (pd)		PERCENT FINES
dPH∥	Station: 59+00		Ë	ER!	PLE		ESU	TEST TYPE	RESS ENG1 (sf)	STRAIN (%)	MAT	- - - - - - - - - - - - - - - - - - -	LL-PL-PI	
GR/	otation. 65 100		В	WAT	SAM	쁜	₽	rest	STRE	STR/	<sup>2</sup> 0	NE PE		ERC
	DEPTH 0.6 <b>ASPHALT - 7"</b>			- 0	0,				8"	0,				<u> </u>
	1.3 ASPHALT TREATED BASE COARSE - 8"													
	LEAN CLAY (CL), brown, medium stiff													
			-		X	3-4 N=					22			77
				_		IN-	-9							
	4.0					1	1				23	99		84
5	LEAN TO FAT CLAY (CL/CH), brown to tan, s	stiff				'	1				23	99		04
E C			5 -			3-3	2.7							
				_	IX	N=					24			78
						5-6	2.7							
5	8.0				X	N=					26			
2 /////	Boring Terminated at 8 Feet		•											
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	Stratification lines are approximate. In-situ, the transition ma	av he gradual					Hammo	r Tvn	e: Autom	atic				
¥	Cadamoduon inico die approximate. Il Politi, tile transition lik	y Do gradual.					i iailiille	ıyp	o. Autoill					
	ncement Method:	See Exhibit A-3 for	descri	iption o	f field		Notes:							
≐  Ho ⊇	Ilow Stem Auger	procedures.												
, Al		See Appendix B for procedures and ad	ditiona	ıl data (	if any	).								
	donment Method: rings backfilled with soil cuttings upon completion.	See Appendix C fo abbreviations.	r expla	anation	of syr	mbols and								
3 1	WATER LEVEL OBSERVATIONS	77					Boring Sta	arted:	04-26-20	18	Borin	ng Com	pleted: 04-26-	2018
Ž		lier	6	30		חנ	Drill Rig: (	CME 7	75		Drille	er: EDI		
<u> </u>				ins St N		· <u>-</u>	Project No					hit· /	N_30	

### **GENERAL NOTES**

#### **DESCRIPTION OF SYMBOLS AND ABBREVIATIONS**

						Water Initially Encountered		(HP)	Hand Penetrometer
	Auger	Shelby Tube	Split Spoon		<u> </u>	Water Level After a Specified Period of Time		(T)	Torvane
<u>ග</u>	Ш		M	/EL		Water Level After a Specified Period of Time	STS	(b/f)	Standard Penetration Test (blows per foot)
PLIN	Rock Core	Macro Core	Modified California Ring Sampler	R LEVE		s indicated on the soil boring levels measured in the	D TE	N	N value
SAMPL	m	$\square$		ATEI	borehole at	the times indicated. er level variations will occur		(PID)	Photo-lonization Detector
	Grab Sample	No Recovery [	Modified Dames & Moore Ring Sampler	>	accurate de levels is not	n low permeability soils, termination of groundwater possible with short term observations.		(OVA)	Organic Vapor Analyzer

#### **DESCRIPTIVE SOIL CLASSIFICATION**

Soil classification is based on the Unified Soil Classification System. Coarse Grained Soils have more than 50% of their dry weight retained on a #200 sieve; their principal descriptors are: boulders, cobbles, gravel or sand. Fine Grained Soils have less than 50% of their dry weight retained on a #200 sieve; they are principally described as clays if they are plastic, and silts if they are slightly plastic or non-plastic. Major constituents may be added as modifiers and minor constituents may be added according to the relative proportions based on grain size. In addition to gradation, coarse-grained soils are defined on the basis of their in-place relative density and fine-grained soils on the basis of their consistency.

#### **LOCATION AND ELEVATION NOTES**

Unless otherwise noted, Latitude and Longitude are approximately determined using a hand-held GPS device. The accuracy of such devices is variable. Surface elevation data annotated with +/- indicates that no actual topographical survey was conducted to confirm the surface elevation. Instead, the surface elevation was approximately determined from topographic maps of the area.

	(More than Density determine	NSITY OF COARSE-GRAI n 50% retained on No. 200 led by Standard Penetration des gravels, sands and sil	sieve.) on Resistance		CONSISTENCY OF FIN (50% or more passing t ency determined by laborato -manual procedures or star	he No. 200 sieve.) bry shear strength testing, t				
RMS	Descriptive Term (Density)	Standard Penetration or N-Value Blows/Ft.	Ring Sampler Blows/Ft.	Descriptive Term (Consistency)	Unconfined Compressive Strength, Qu, psf	Standard Penetration or N-Value Blows/Ft.	Ring Sampler Blows/Ft.			
ᆵ	Very Loose	0 - 3	0 - 6	Very Soft	less than 500	0 - 1	< 3			
NGT	Loose	4 - 9	7 - 18	Soft	500 to 1,000	2 - 4	3 - 4			
STREN	Medium Dense	10 - 29	19 - 58	Medium-Stiff	1,000 to 2,000	4 - 8	5 - 9			
ြင	Dense	30 - 50	59 - 98	Stiff	2,000 to 4,000	8 - 15	10 - 18			
	Very Dense	> 50	<u>≥</u> 99	Very Stiff	4,000 to 8,000	15 - 30	19 - 42			
	Hard > 8,000 > 30 > 42									

#### **RELATIVE PROPORTIONS OF SAND AND GRAVEL**

<u>Descriptive Term(s)</u>	Percent of	<u>Major Component</u>	Particle Size
of other constituents	Dry Weight	<u>of Sample</u>	
Trace With Modifier	< 15 15 - 29 > 30	Boulders Cobbles Gravel Sand Silt or Clay	Over 12 in. (300 mm) 12 in. to 3 in. (300mm to 75mm) 3 in. to #4 sieve (75mm to 4.75 mm) #4 to #200 sieve (4.75mm to 0.075mm Passing #200 sieve (0.075mm)

**GRAIN SIZE TERMINOLOGY** 

PLASTICITY DESCRIPTION

#### **RELATIVE PROPORTIONS OF FINES**

<u>Descriptive Term(s)</u> of other constituents	Percent of Dry Weight	<u>Term</u>	Plasticity Index
or other constituents	Diy Woight	Non-plastic	0
Trace	< 5	Low	1 - 10
With	5 - 12	Medium	11 - 30
Modifier	> 12	High	> 30



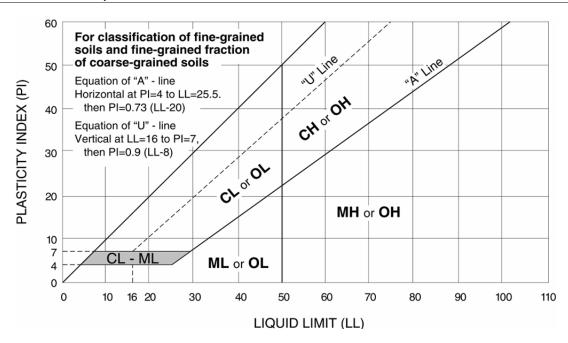
### UNIFIED SOIL CLASSIFICATION SYSTEM

				8	Soil Classification
Criteria for Assign	ning Group Symbols	and Group Names	s Using Laboratory Tests <sup>A</sup>	Group Symbol	Group Name <sup>B</sup>
	Gravels:	Clean Gravels:	$Cu \ge 4$ and $1 \le Cc \le 3^E$	GW	Well-graded gravel F
	More than 50% of	Less than 5% fines <sup>C</sup>	Cu < 4 and/or 1 > Cc > 3 <sup>E</sup>	GP	Poorly graded gravel F
	coarse fraction retained	Gravels with Fines:	Fines classify as ML or MH	GM	Silty gravel F,G,H
Coarse Grained Soils: More than 50% retained	on No. 4 sieve	More than 12% fines <sup>C</sup>	Fines classify as CL or CH	GC	Clayey gravel F,G,H
on No. 200 sieve	Sands:	Clean Sands:	Cu ≥ 6 and 1 ≤ Cc ≤ 3 <sup>E</sup>	SW	Well-graded sand I
011110. 200 01010	50% or more of coarse	Less than 5% fines D	Cu < 6 and/or 1 > Cc > 3 <sup>E</sup>	SP	Poorly graded sand I
	fraction passes No. 4	Sands with Fines:	Fines classify as ML or MH	SM	Silty sand G,H,I
	sieve	More than 12% fines D	Fines classify as CL or CH	SC	Clayey sand G,H,I
		Inorganic:	PI > 7 and plots on or above "A" line J	CL	Lean clay K,L,M
	Silts and Clays:	morganic.	PI < 4 or plots below "A" line J	ML	Silt K,L,M
	Liquid limit less than 50	Organia	Liquid limit - oven dried	OL	Organic clay K,L,M,N
Fine-Grained Soils: 50% or more passes the		Organic:	Liquid limit - not dried	OL	Organic silt K,L,M,O
No. 200 sieve		Inorgania	PI plots on or above "A" line	CH	Fat clay <sup>K,L,M</sup>
	Silts and Clays:	Inorganic:	PI plots below "A" line	МН	Elastic Silt K,L,M
	Liquid limit 50 or more	Organic:	Liquid limit - oven dried < 0.75	ОН	Organic clay K,L,M,P
		Organic.	Liquid limit - not dried < 0.75	ОП	Organic silt K,L,M,Q
Highly organic soils:	Primarily	organic matter, dark in o	color, and organic odor	PT	Peat

<sup>&</sup>lt;sup>A</sup> Based on the material passing the 3-inch (75-mm) sieve

<sup>E</sup> Cu = 
$$D_{60}/D_{10}$$
 Cc =  $\frac{(D_{30})^2}{D_{10} \times D_{60}}$ 

Q PI plots below "A" line.





<sup>&</sup>lt;sup>B</sup> If field sample contained cobbles or boulders, or both, add "with cobbles or boulders, or both" to group name.

<sup>&</sup>lt;sup>c</sup> Gravels with 5 to 12% fines require dual symbols: GW-GM well-graded gravel with silt, GW-GC well-graded gravel with clay, GP-GM poorly graded gravel with silt, GP-GC poorly graded gravel with clay.

D Sands with 5 to 12% fines require dual symbols: SW-SM well-graded sand with silt, SW-SC well-graded sand with clay, SP-SM poorly graded sand with silt, SP-SC poorly graded sand with clay

 $<sup>^{\</sup>text{F}}$  If soil contains  $\geq$  15% sand, add "with sand" to group name.

<sup>&</sup>lt;sup>G</sup> If fines classify as CL-ML, use dual symbol GC-GM, or SC-SM.

<sup>&</sup>lt;sup>H</sup> If fines are organic, add "with organic fines" to group name.

<sup>&</sup>lt;sup>1</sup> If soil contains ≥ 15% gravel, add "with gravel" to group name.

If Atterberg limits plot in shaded area, soil is a CL-ML, silty clay.

K If soil contains 15 to 29% plus No. 200, add "with sand" or "with gravel," whichever is predominant.

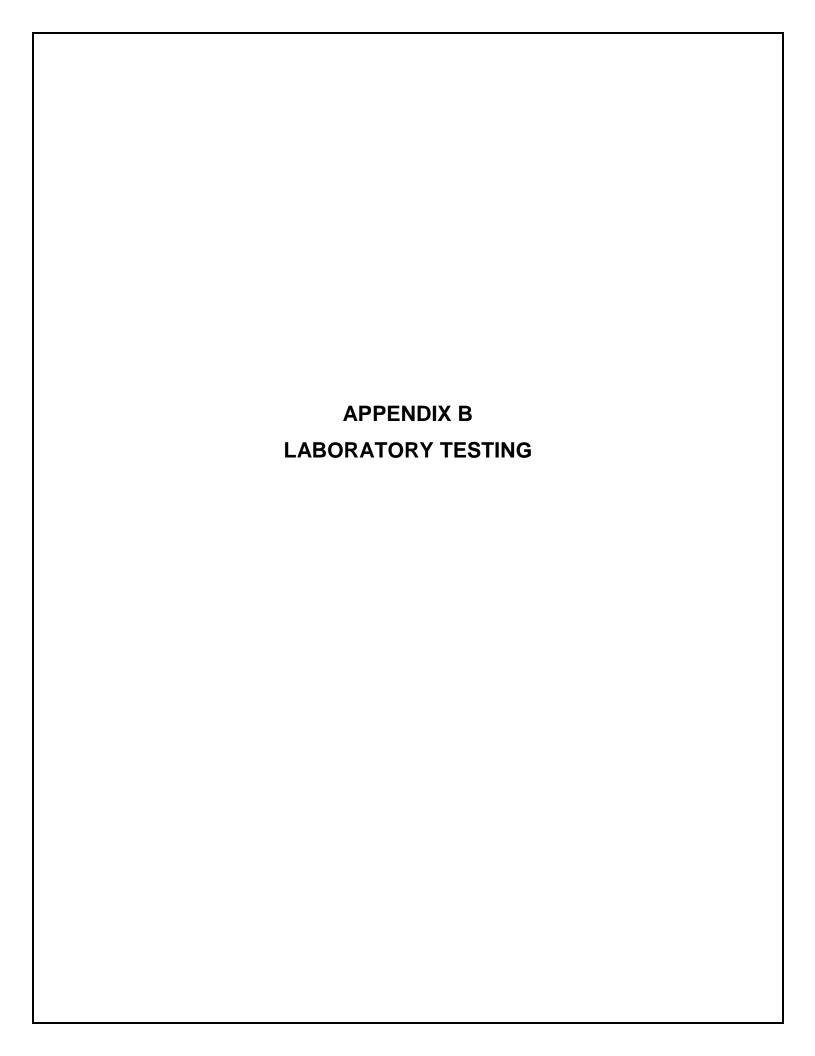
 $<sup>^{\</sup>text{L}}$  If soil contains  $\geq$  30% plus No. 200 predominantly sand, add "sandy" to group name.

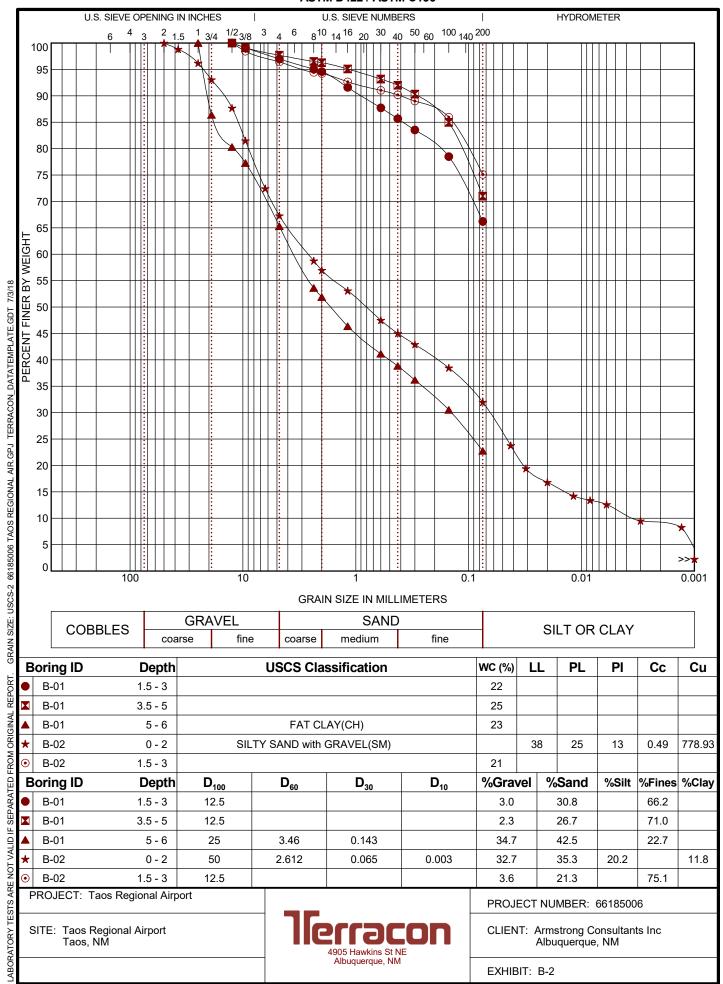
M If soil contains ≥ 30% plus No. 200, predominantly gravel, add "gravelly" to group name.

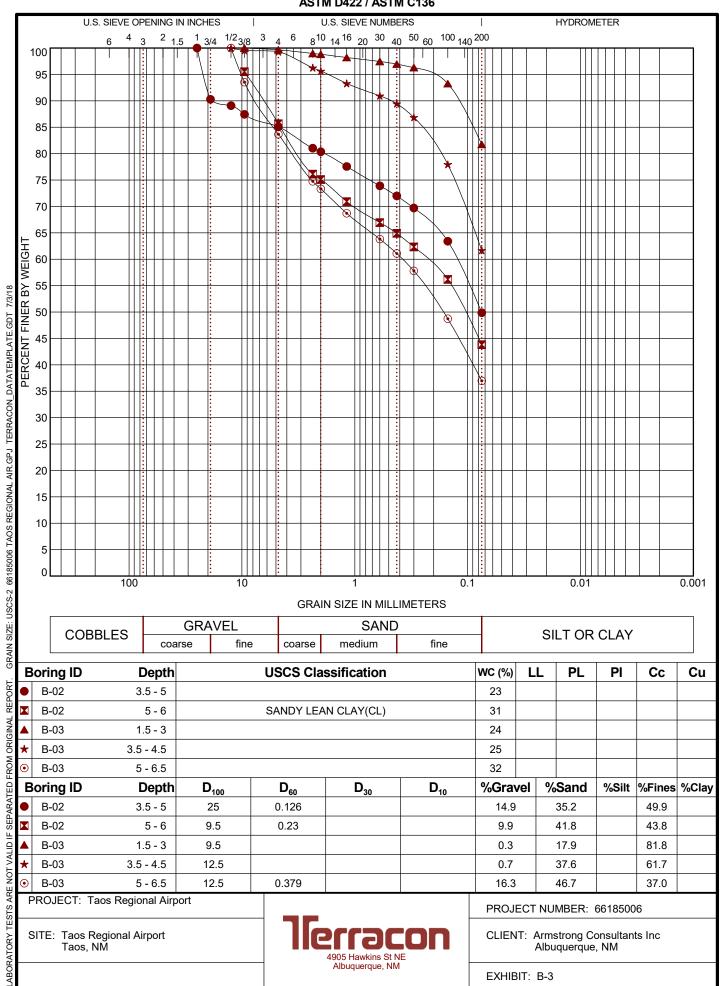
 $<sup>^{</sup>N}$  PI  $\geq$  4 and plots on or above "A" line.

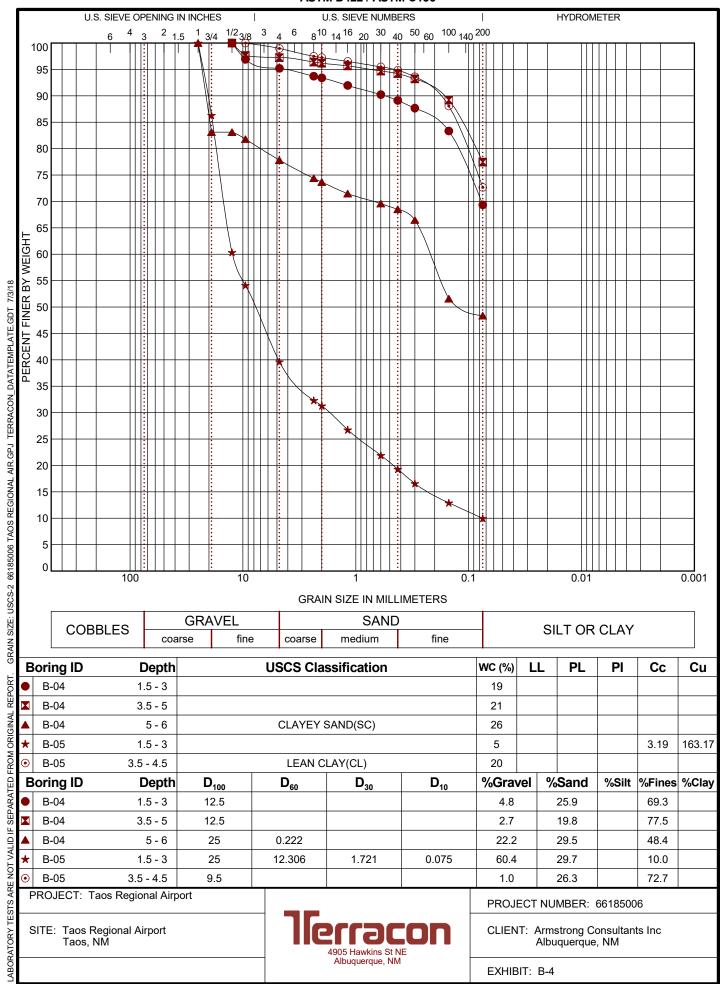
 $<sup>^{\</sup>text{O}}$  PI < 4 or plots below "A" line.

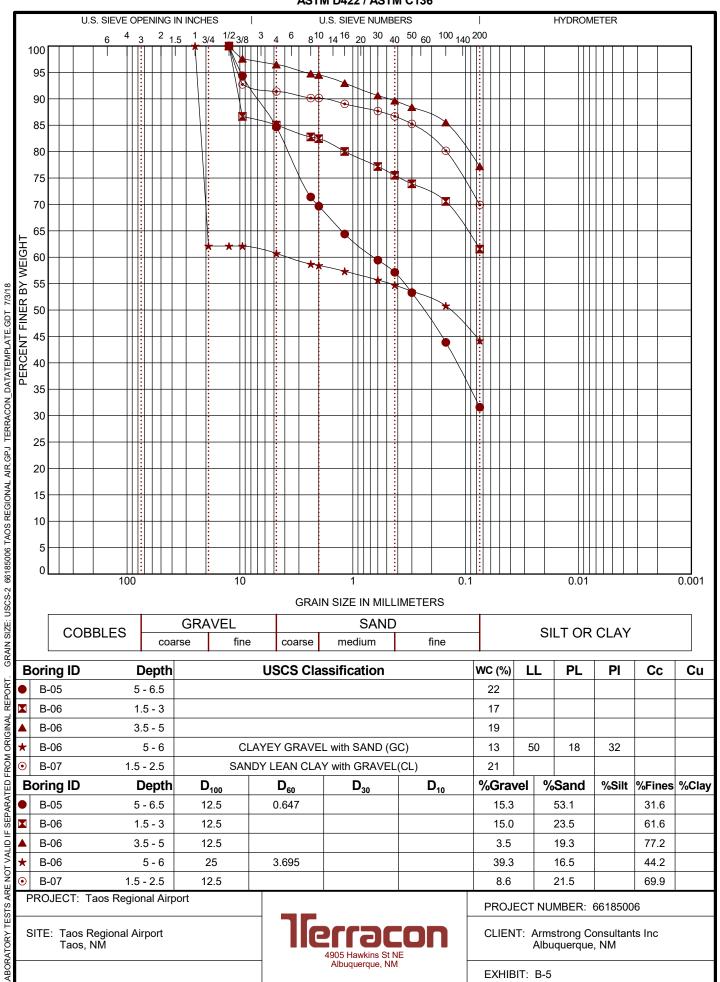
P PI plots on or above "A" line.

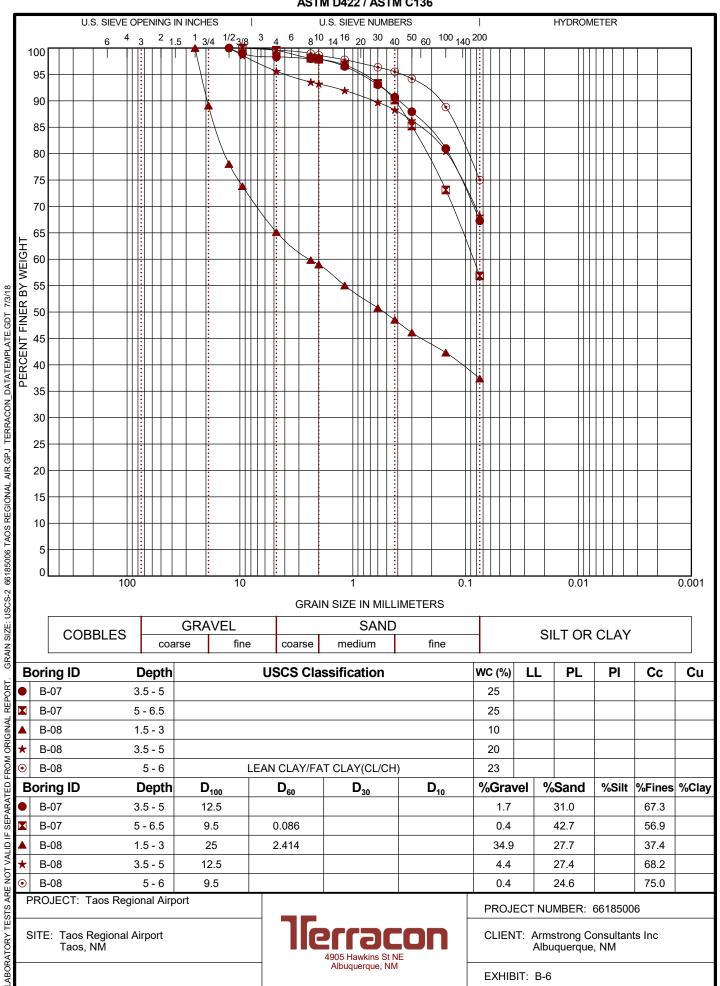


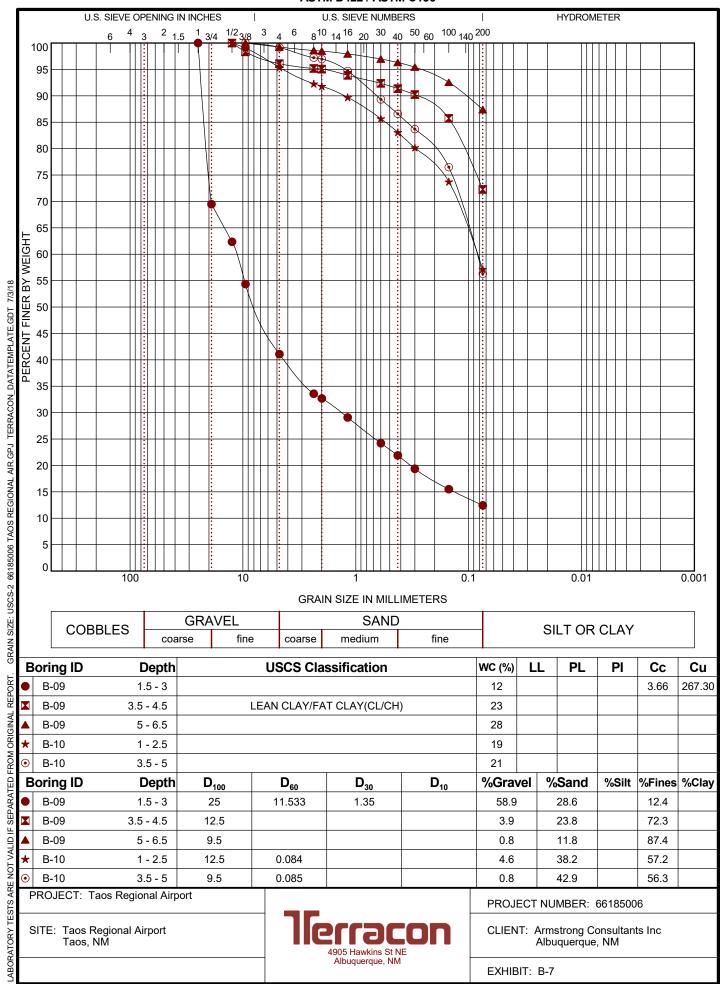


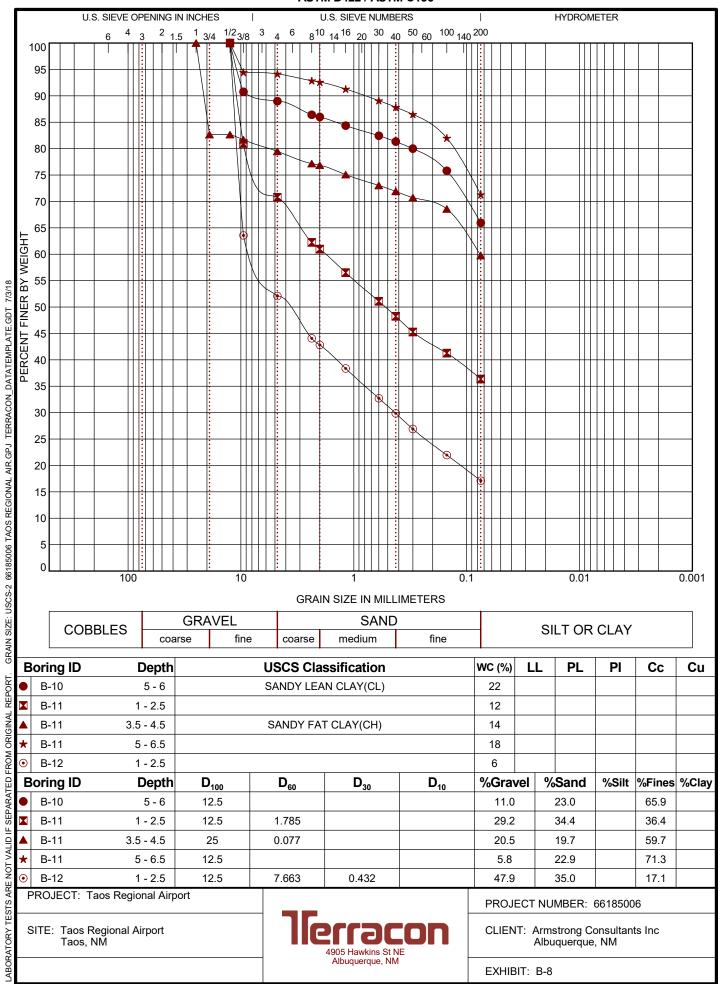


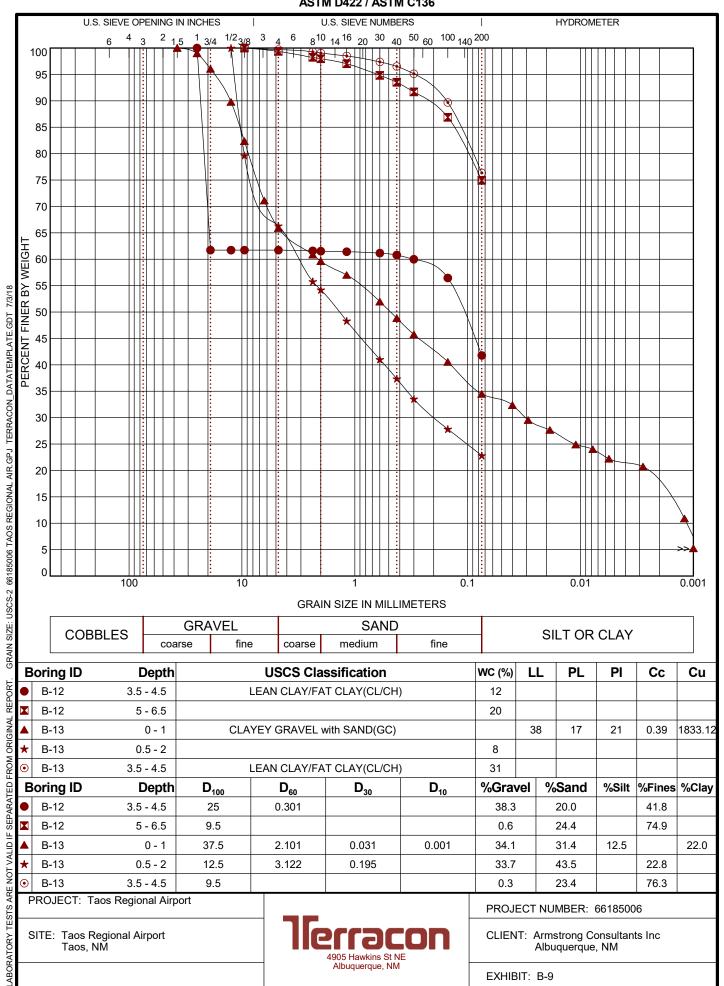


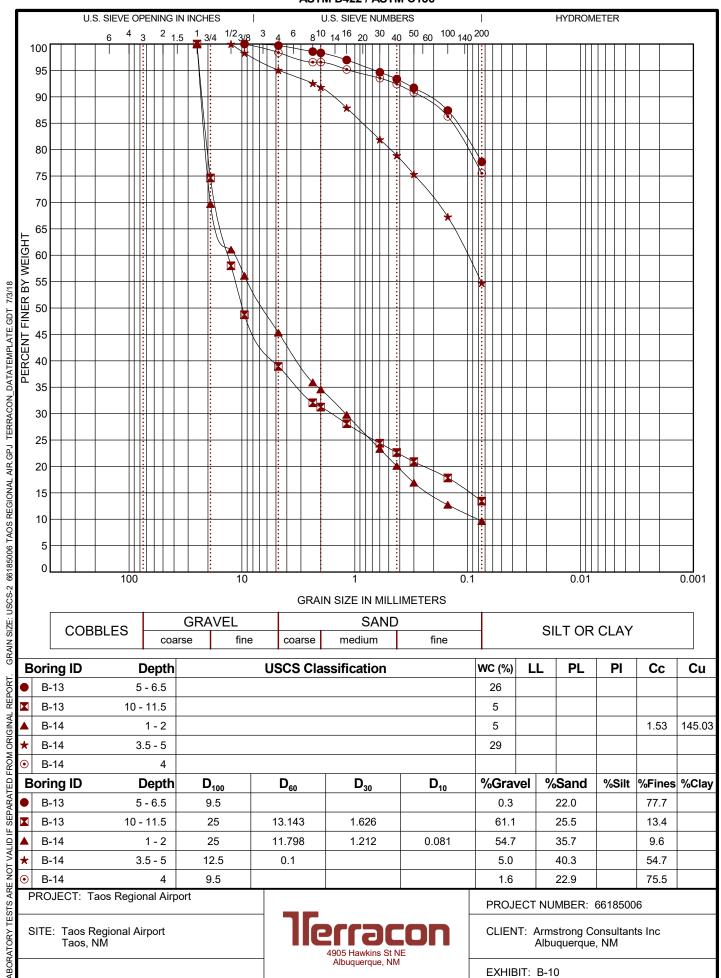


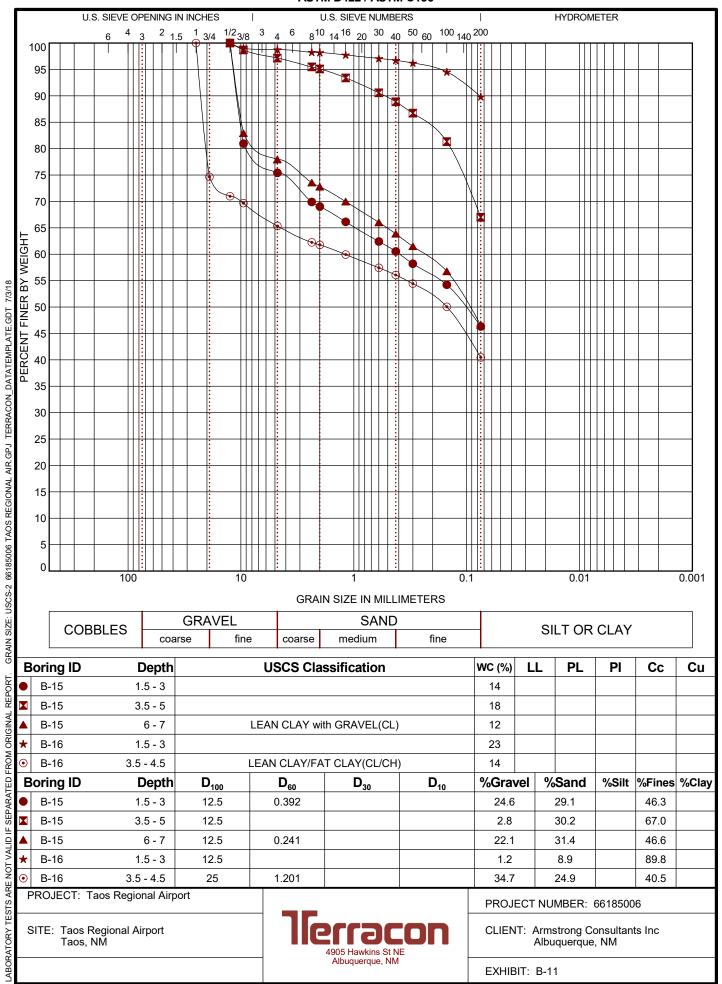


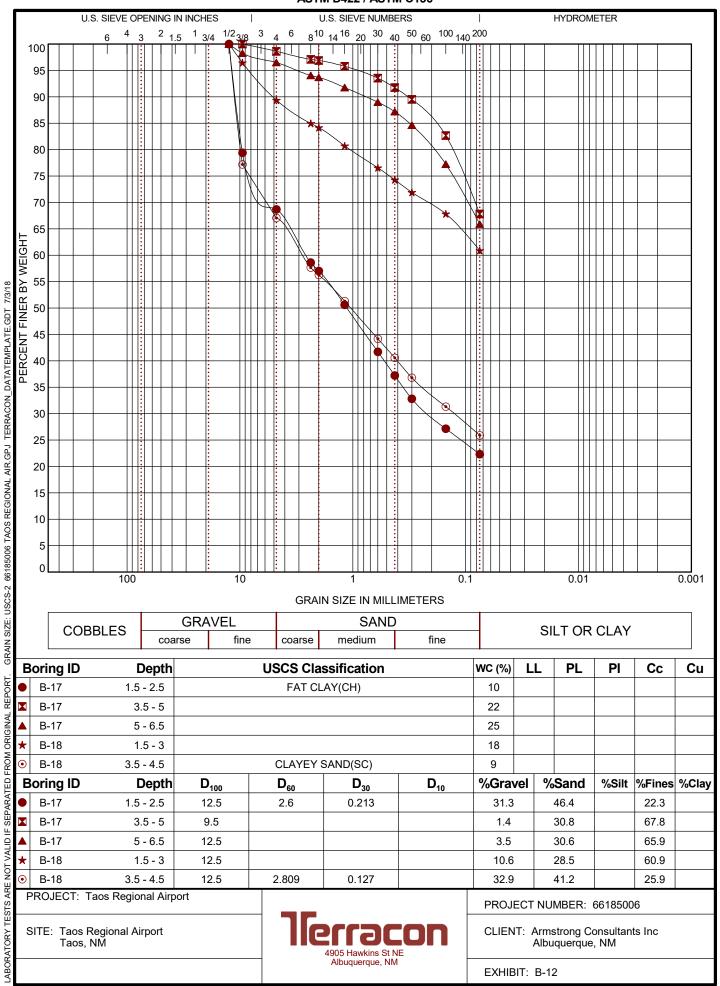


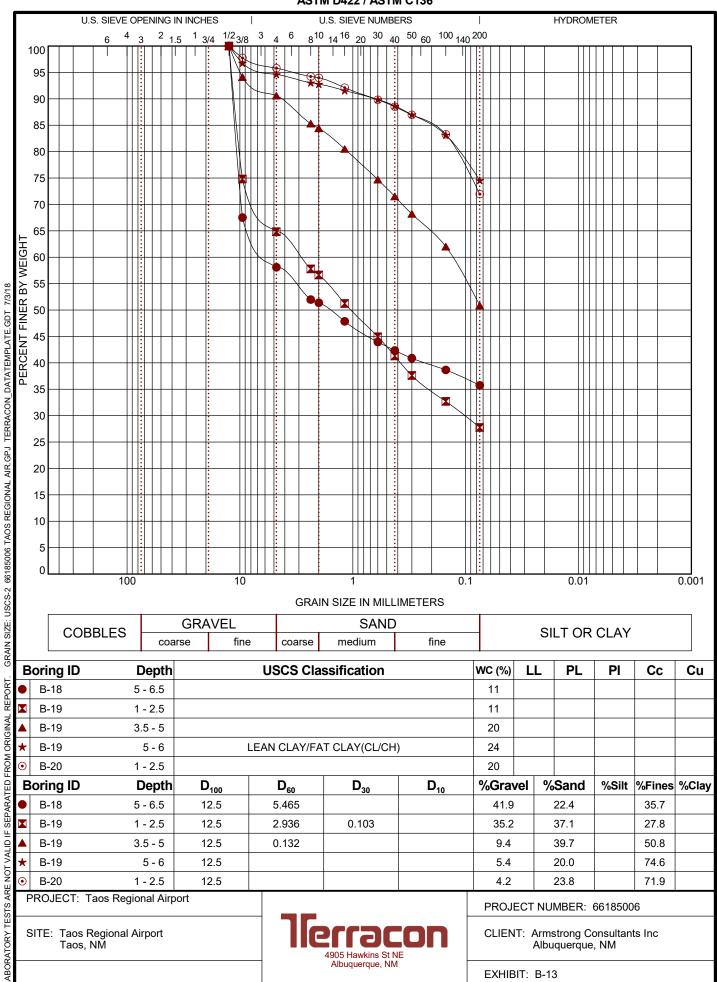


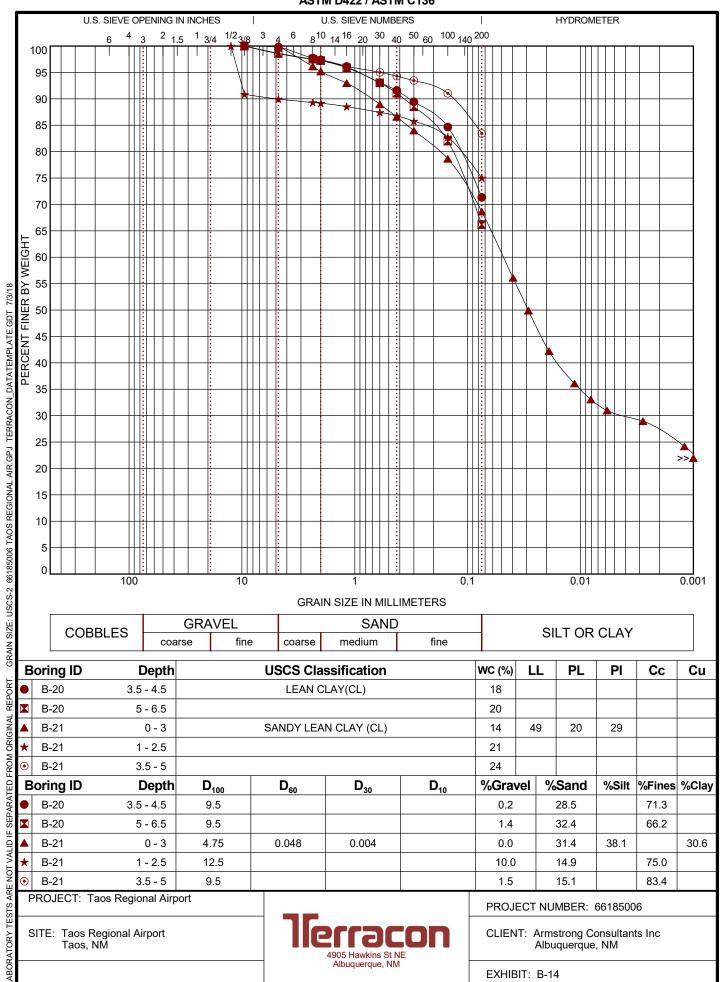


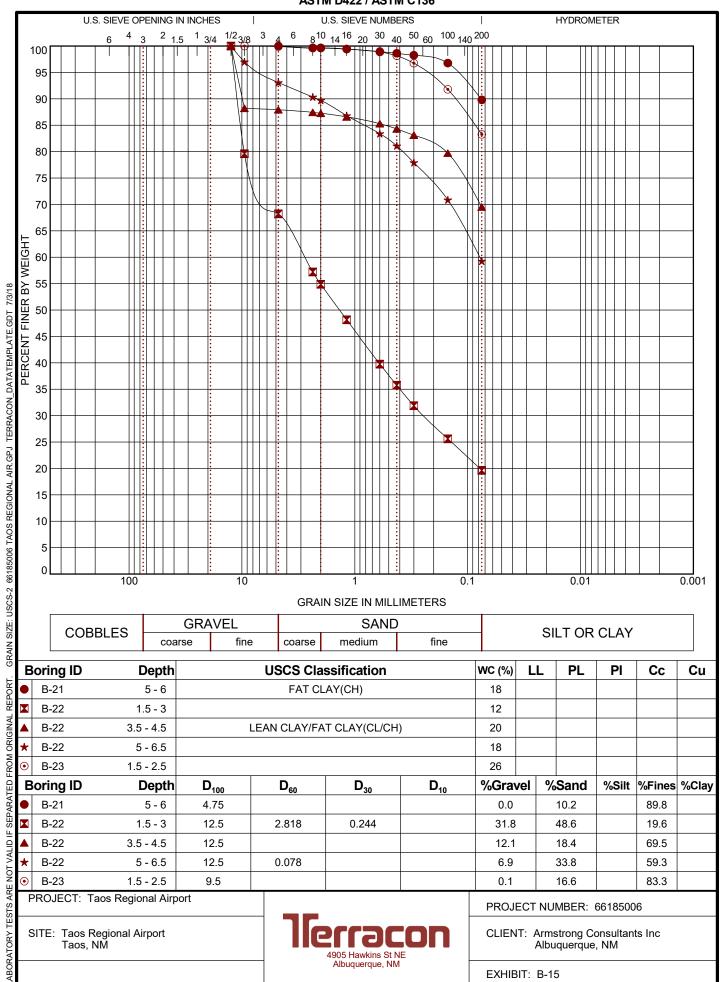


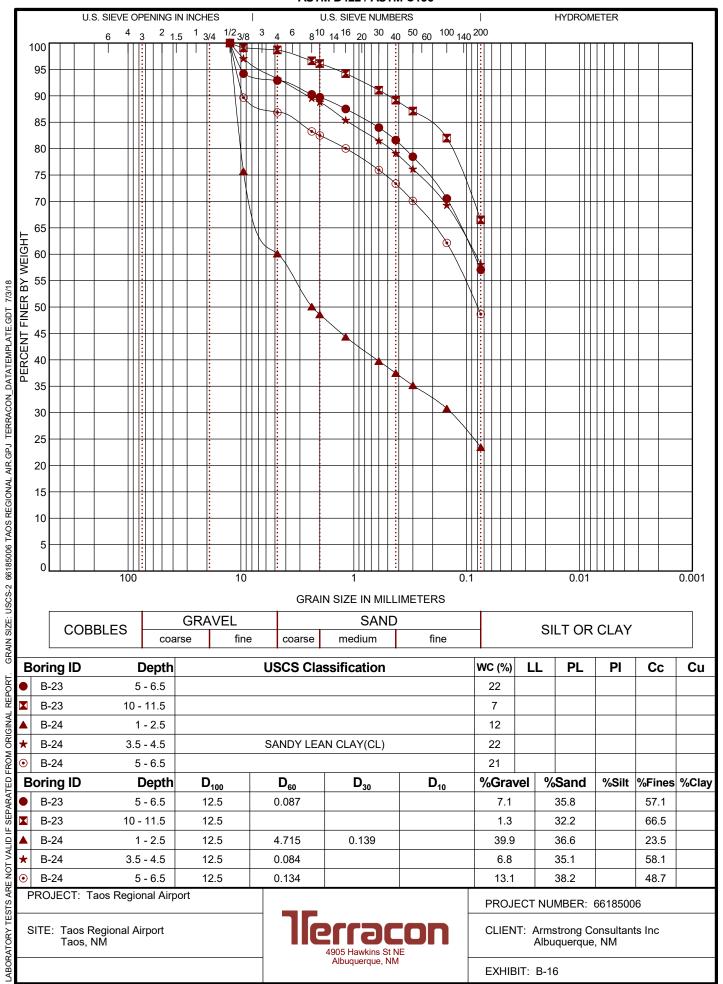


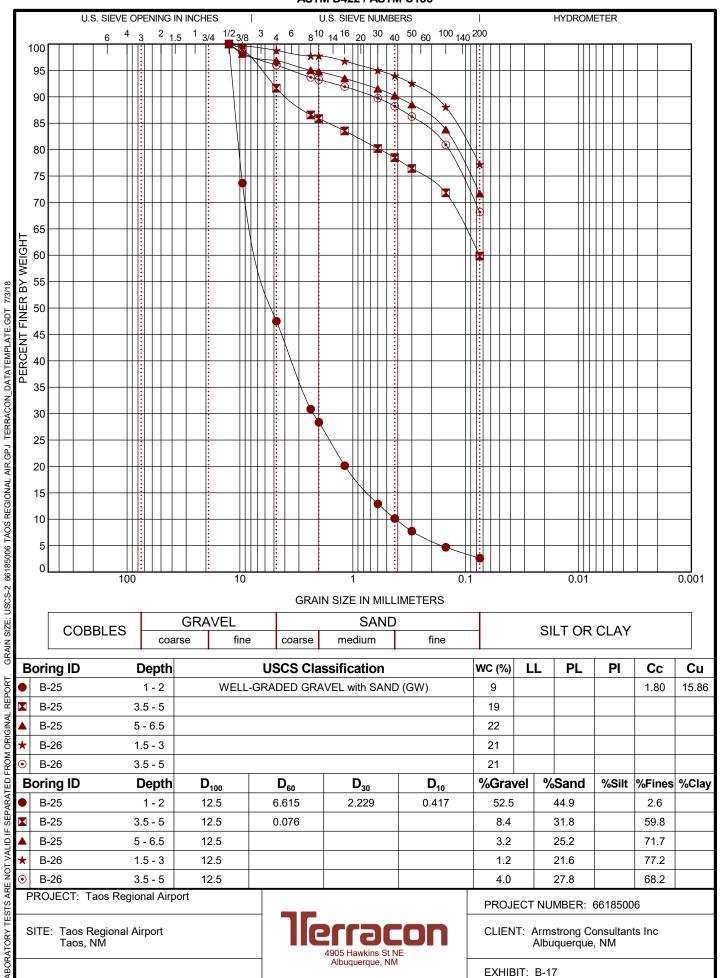


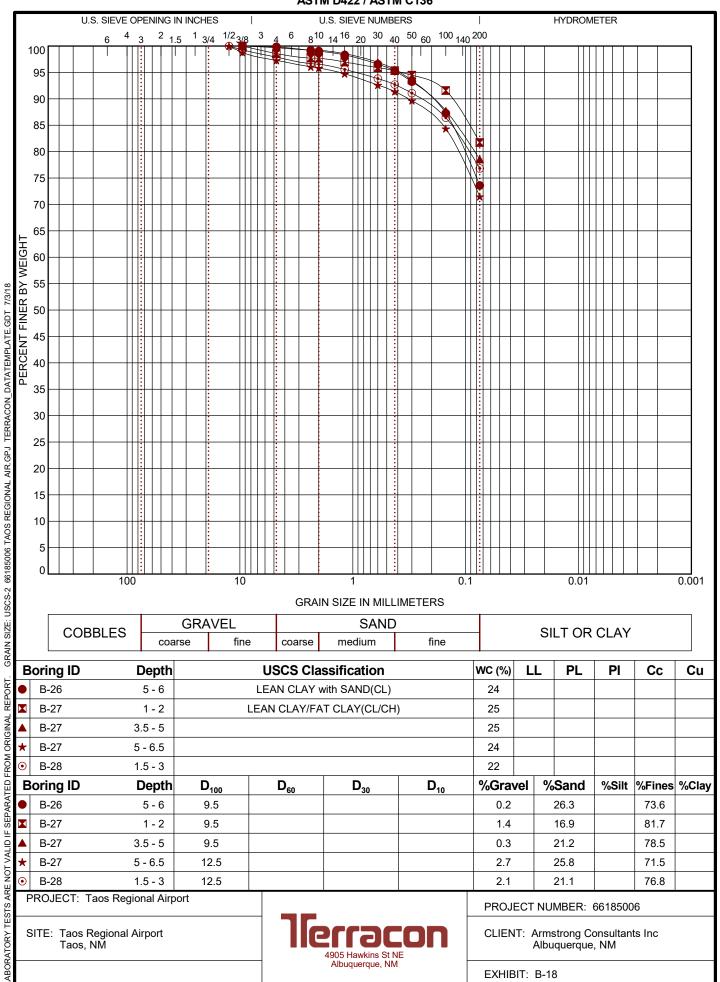


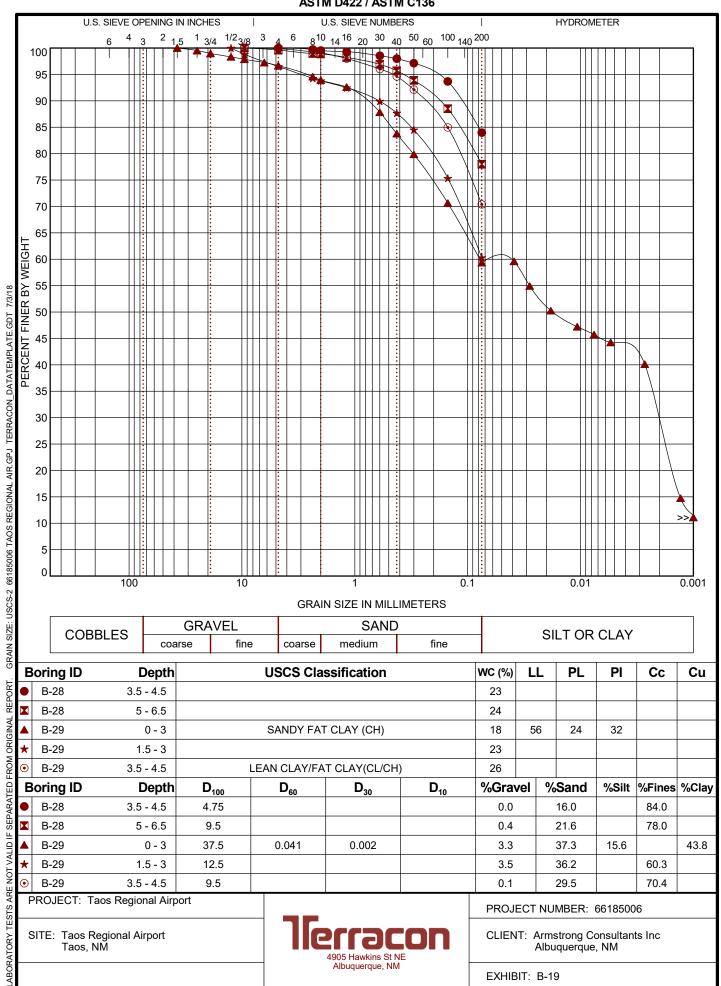


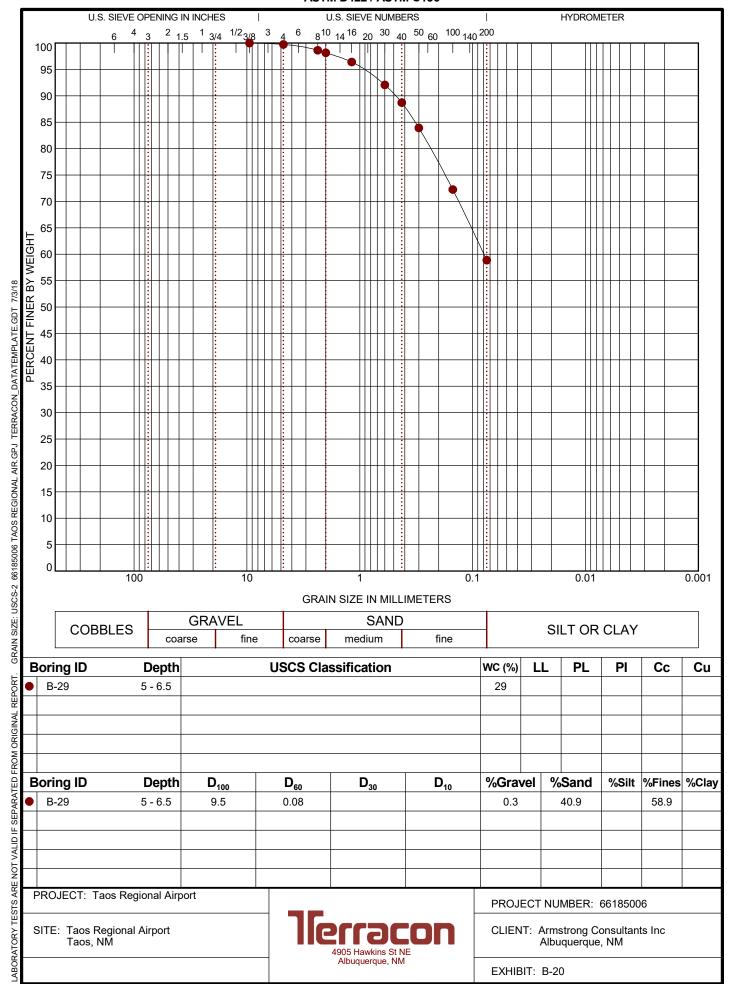




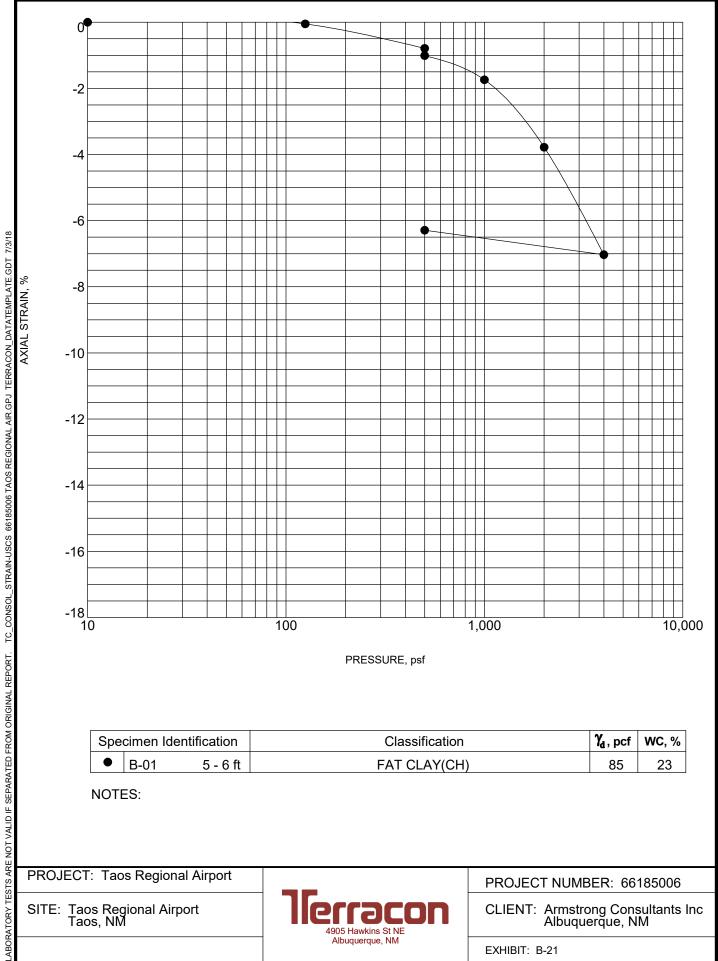








**ASTM D2435** 



PRESSURE, psf
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Spe	Specimen Identification		Classification	$\gamma_d$ , pcf	WC, %
•	B-01	5 - 6 ft	FAT CLAY(CH)	85	23

NOTES:

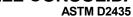
PROJECT: Taos Regional Airport

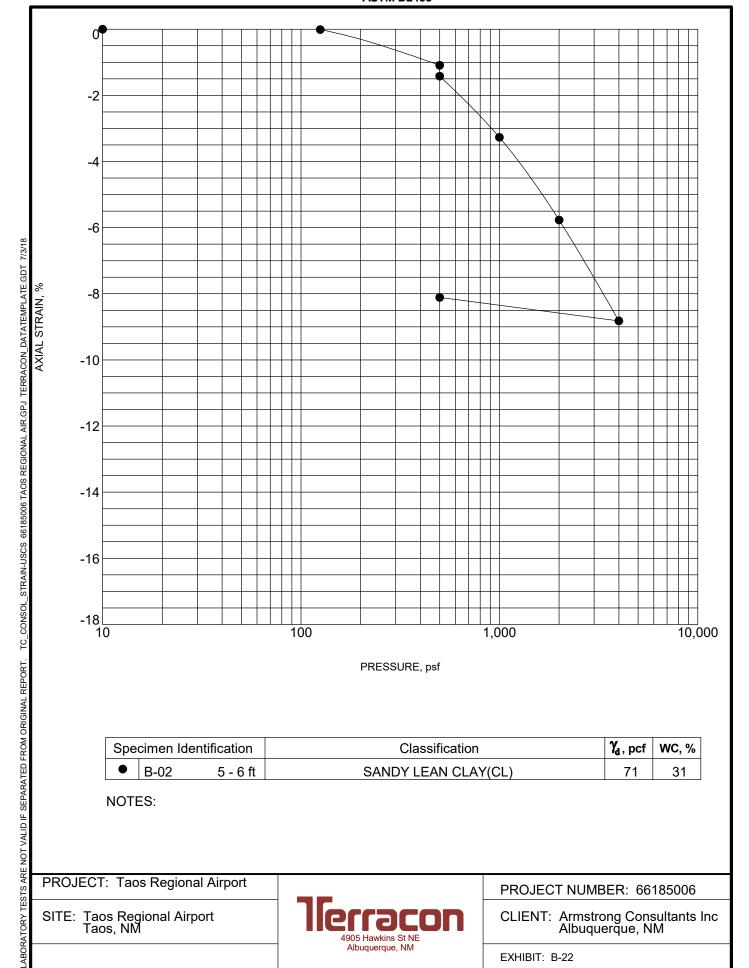
SITE: Taos Regional Airport Taos, NM



PROJECT NUMBER: 66185006

CLIENT: Armstrong Consultants Inc Albuquerque, NM





Spe	cimen Ide	entification	Classification	$\gamma_{d}$ , pcf	WC, %
•	B-02	5 - 6 ft	SANDY LEAN CLAY(CL)	71	31

NOTES:

PROJECT: Taos Regional Airport

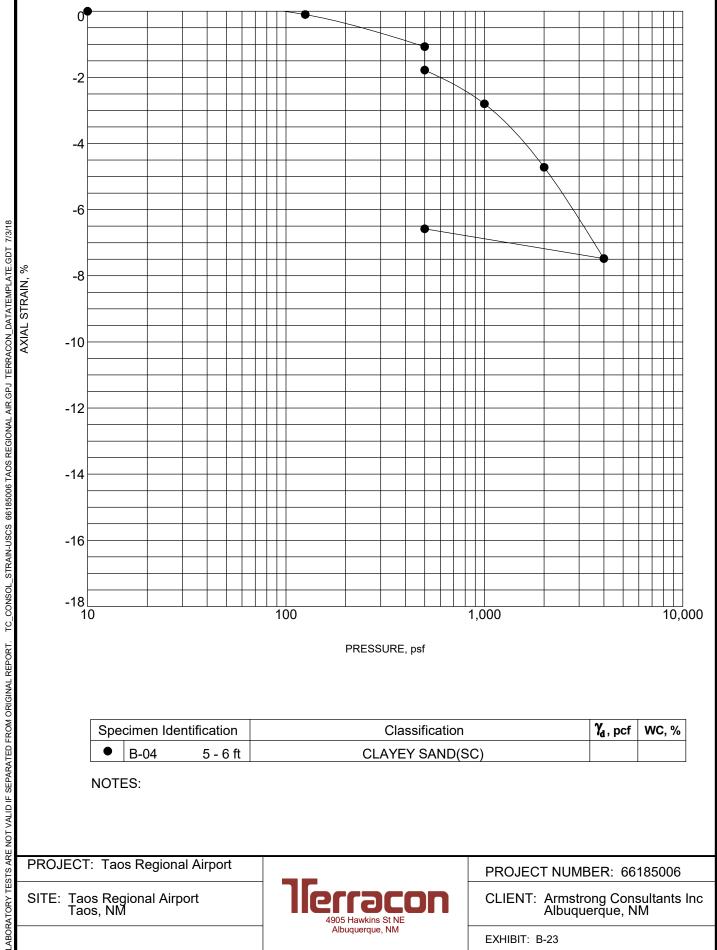
SITE: Taos Regional Airport Taos, NM



PROJECT NUMBER: 66185006

CLIENT: Armstrong Consultants Inc Albuquerque, NM





PRESSURE, psf

Spe	cimen Ide	entification	Classification	$\gamma_d$ , pcf	WC, %
•	B-04	5 - 6 ft	CLAYEY SAND(SC)		

NOTES:

PROJECT: Taos Regional Airport

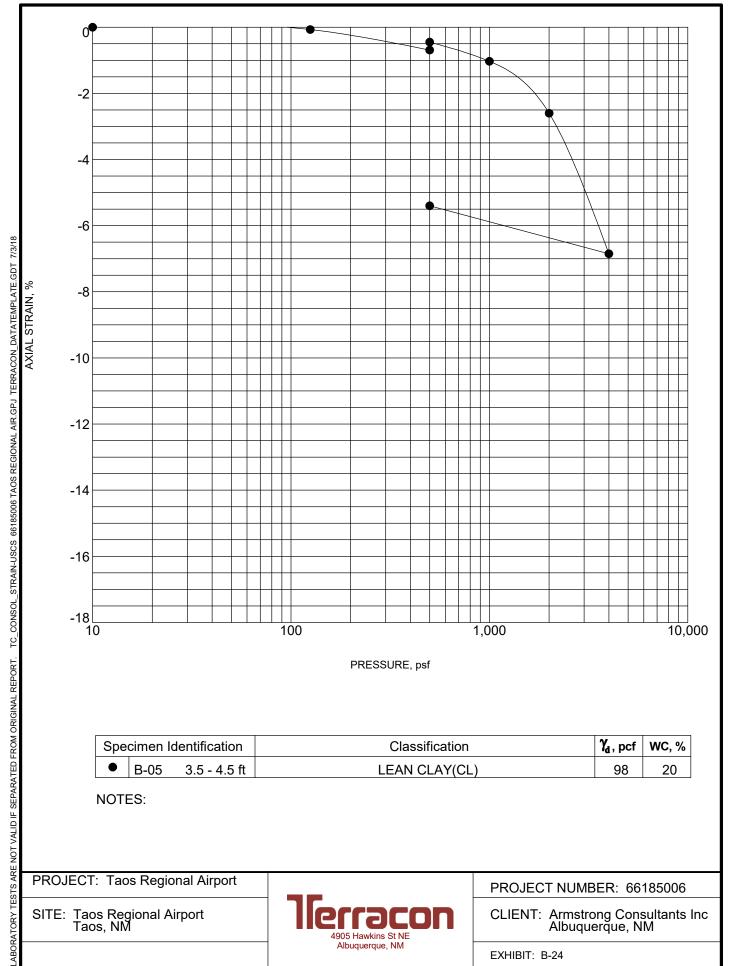
SITE: Taos Regional Airport Taos, NM



PROJECT NUMBER: 66185006

CLIENT: Armstrong Consultants Inc Albuquerque, NM

**ASTM D2435** 



Spec	cimen Identification	Classification	$\gamma_{\!d}$ , pcf	WC, %

LEAN CLAY(CL)

NOTES:

B-05

3.5 - 4.5 ft

PROJECT: Taos Regional Airport

SITE: Taos Regional Airport Taos, NM



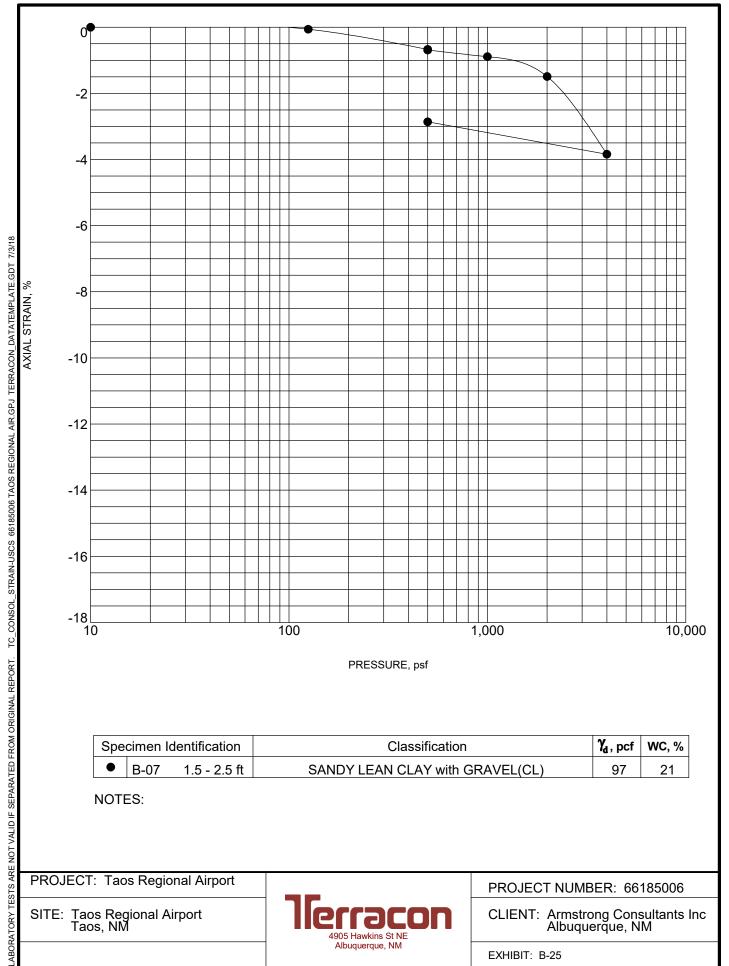
PROJECT NUMBER: 66185006

CLIENT: Armstrong Consultants Inc Albuquerque, NM

98

20

**ASTM D2435** 



Specimen Identification			Classification	$\gamma_d$ , pcf	WC, %
•	B-07	1.5 - 2.5 ft	SANDY LEAN CLAY with GRAVEL(CL)	97	21

NOTES:

PROJECT: Taos Regional Airport

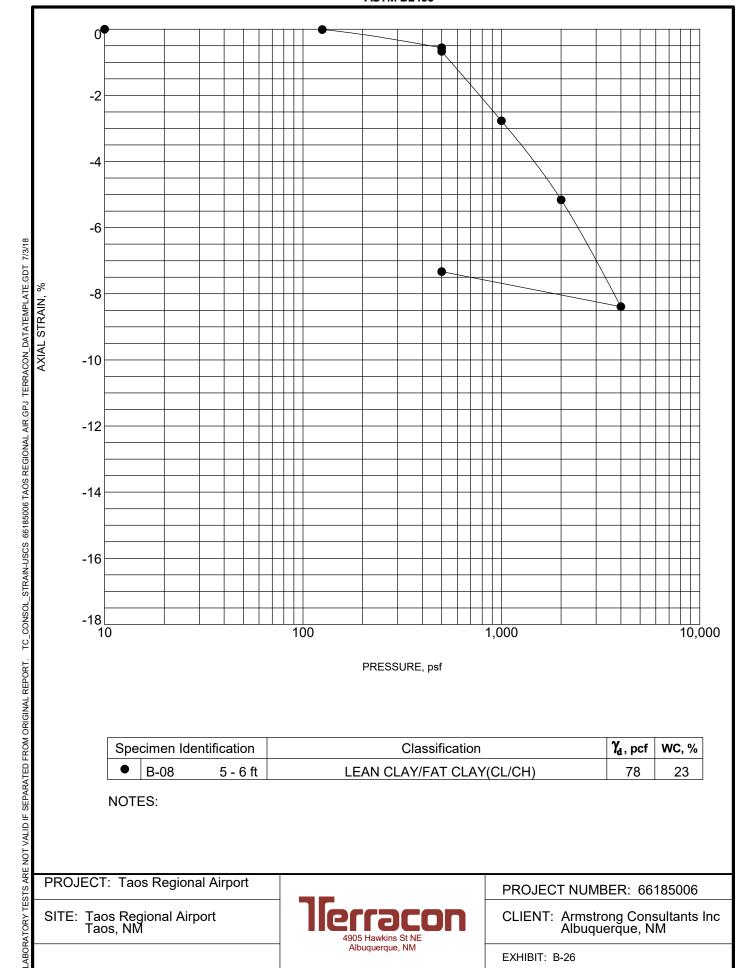
SITE: Taos Regional Airport Taos, NM



PROJECT NUMBER: 66185006

CLIENT: Armstrong Consultants Inc Albuquerque, NM





Specimen Identification		entification	Classification	$\gamma_d$ , pcf	WC, %
•	B-08	5 - 6 ft	LEAN CLAY/FAT CLAY(CL/CH)	78	23

NOTES:

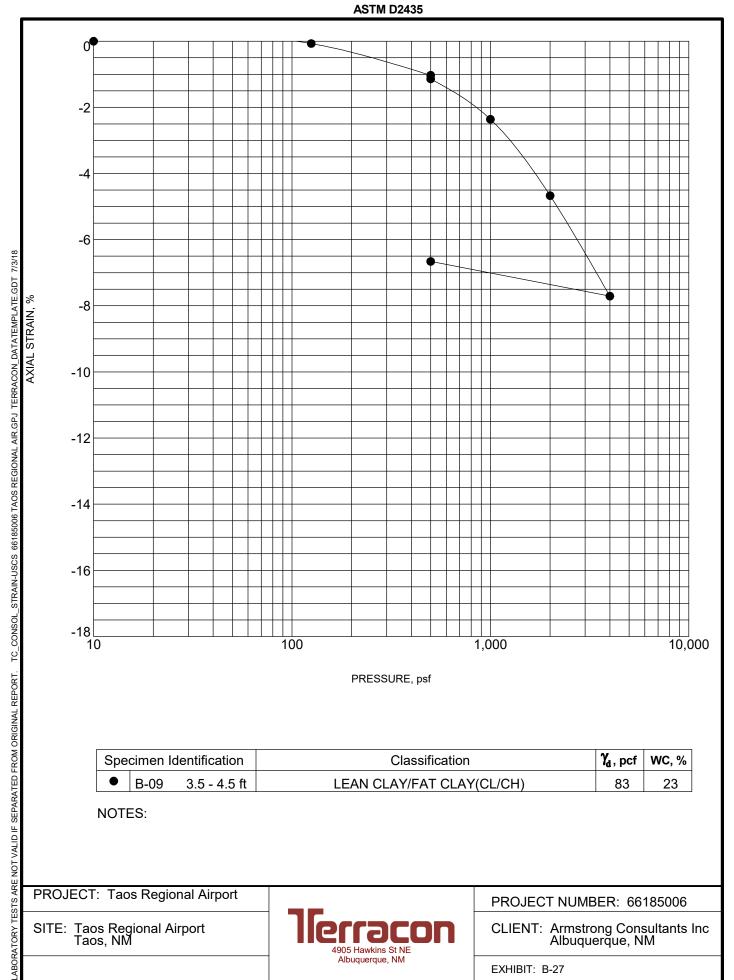
PROJECT: Taos Regional Airport

SITE: Taos Regional Airport Taos, NM



PROJECT NUMBER: 66185006

CLIENT: Armstrong Consultants Inc Albuquerque, NM



Specimen Identif		dentification	Classification	$\gamma_d$ , pcf	WC, %	
	•	B-09	3.5 - 4.5 ft	LEAN CLAY/FAT CLAY(CL/CH)	83	23

NOTES:

PROJECT: Taos Regional Airport

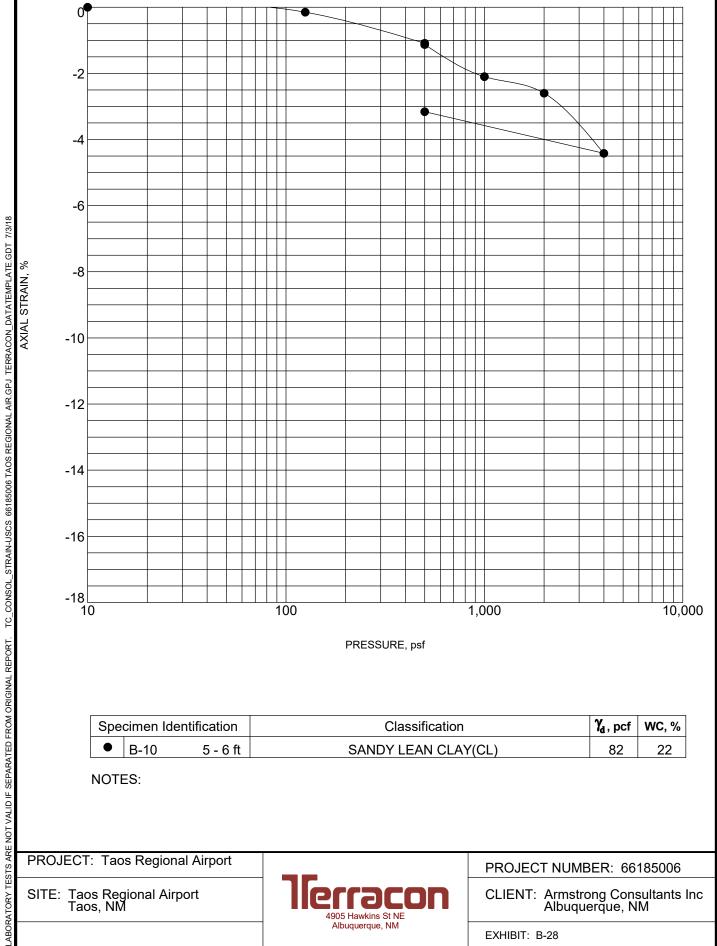
SITE: Taos Regional Airport Taos, NM



PROJECT NUMBER: 66185006

CLIENT: Armstrong Consultants Inc Albuquerque, NM





PRESSURE,	nsf
FINESSUINE.	บอเ

Specimen Identification		entification	Classification	$\gamma_{d}$ , pcf	WC, %
•	B-10	5 - 6 ft	SANDY LEAN CLAY(CL)	82	22

NOTES:

PROJECT: Taos Regional Airport

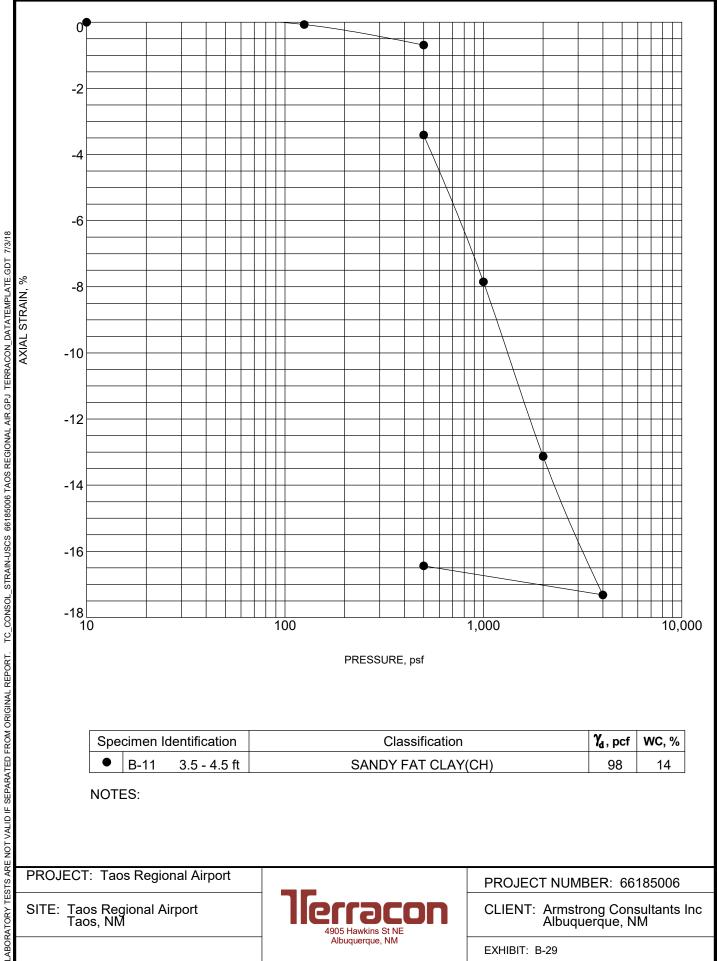
SITE: Taos Regional Airport Taos, NM



PROJECT NUMBER: 66185006

CLIENT: Armstrong Consultants Inc Albuquerque, NM

**ASTM D2435** 



PRESSU	JRE,	psf
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Specimen Identification		Classification	$\gamma_{\!\!d}$ , pcf	WC, %
•	3.5 - 4.5 ft	SANDY FAT CLAY(CH)	98	14

NOTES:

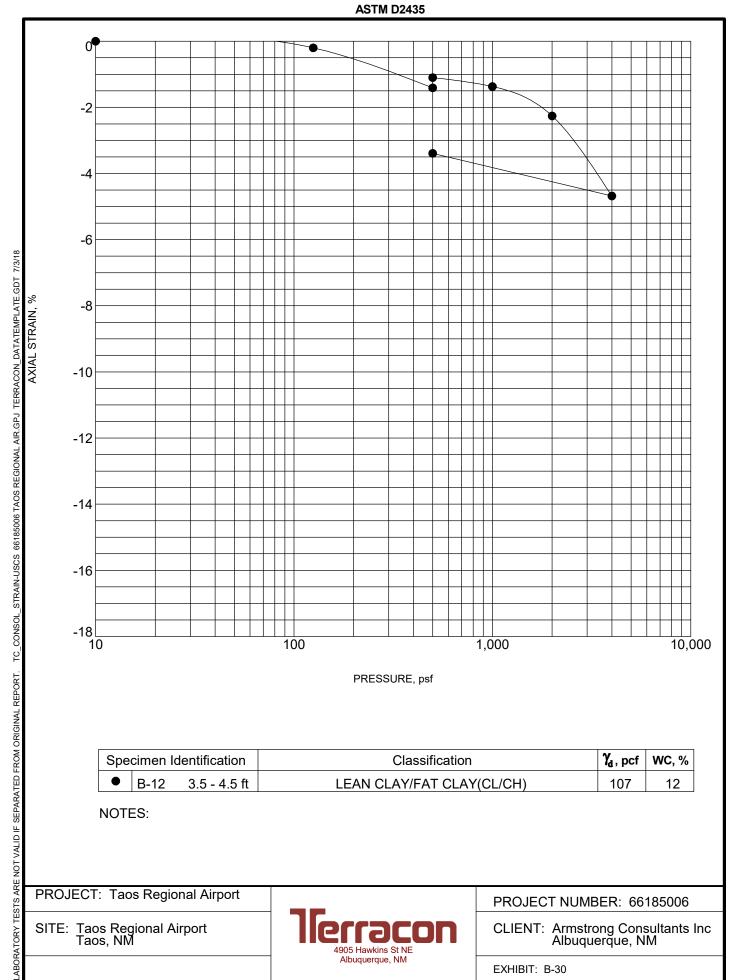
PROJECT: Taos Regional Airport

SITE: Taos Regional Airport Taos, NM



PROJECT NUMBER: 66185006

CLIENT: Armstrong Consultants Inc Albuquerque, NM



Specimen Identification		dentification	Classification	$\gamma_d$ , pcf	WC, %
•	B-12	3.5 - 4.5 ft	LEAN CLAY/FAT CLAY(CL/CH)	107	12

NOTES:

PROJECT: Taos Regional Airport

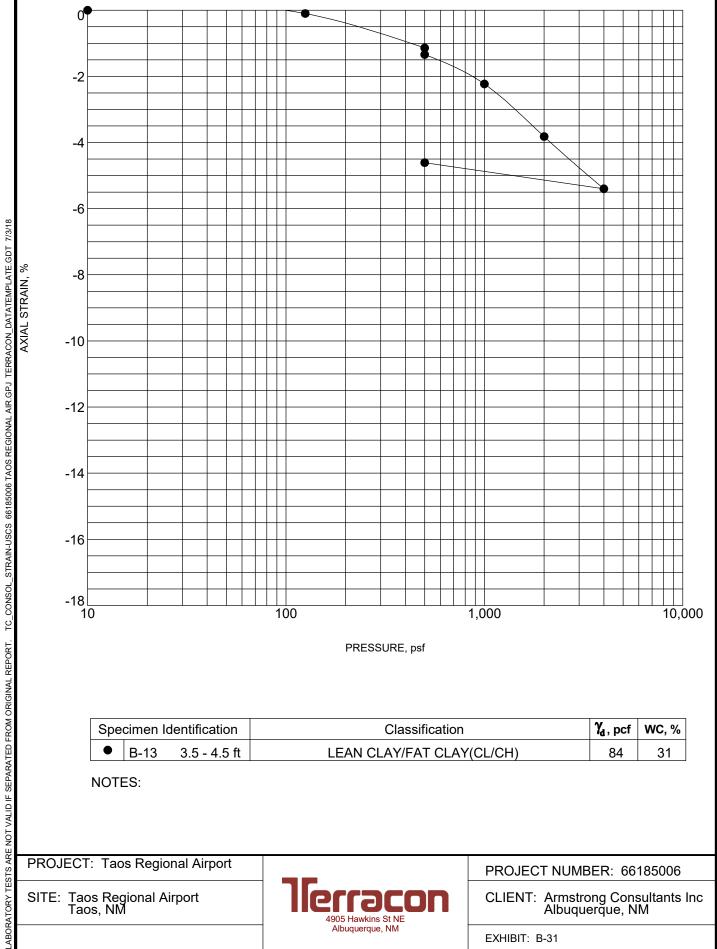
SITE: Taos Regional Airport Taos, NM



PROJECT NUMBER: 66185006

CLIENT: Armstrong Consultants Inc Albuquerque, NM





PRESSU	JRE,	psf
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Specimen Identification		dentification	Classification	$\gamma_{d}$ , pcf	WC, %	
	•	B-13	3.5 - 4.5 ft	LEAN CLAY/FAT CLAY(CL/CH)	84	31

NOTES:

PROJECT: Taos Regional Airport

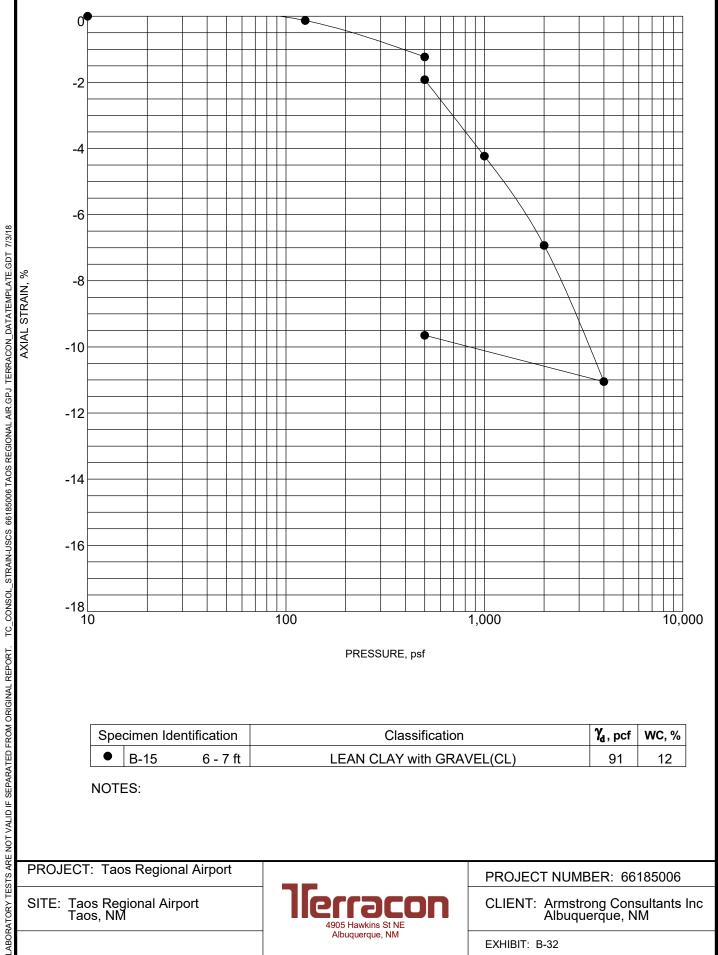
SITE: Taos Regional Airport Taos, NM



PROJECT NUMBER: 66185006

CLIENT: Armstrong Consultants Inc Albuquerque, NM

**ASTM D2435** 



PRESSURE,	nsf
FINESSUIVE.	DOL

Spe		ecimen Identification		Classification	$\gamma_{d}$ , pcf	WC, %
	•	B-15	6 - 7 ft	LEAN CLAY with GRAVEL(CL)	91	12

NOTES:

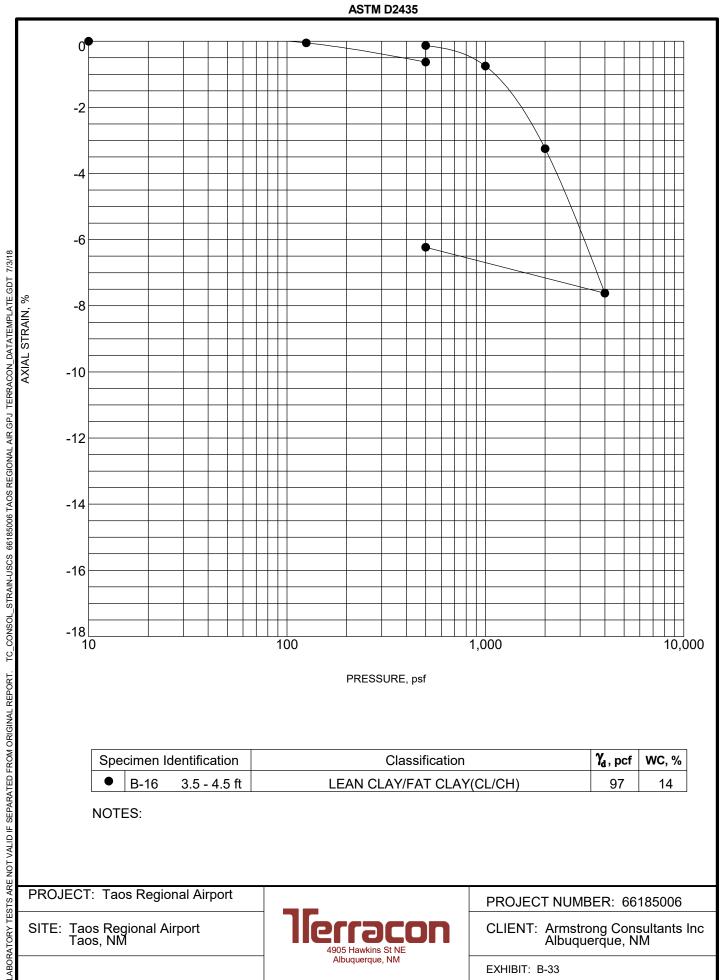
PROJECT: Taos Regional Airport

SITE: Taos Regional Airport Taos, NM



PROJECT NUMBER: 66185006

CLIENT: Armstrong Consultants Inc Albuquerque, NM



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Spe		cimen lo	dentification	Classification	$\gamma_{d}$ , pcf	WC, %
	•	B-16	3.5 - 4.5 ft	LEAN CLAY/FAT CLAY(CL/CH)	97	14

NOTES:

PROJECT: Taos Regional Airport

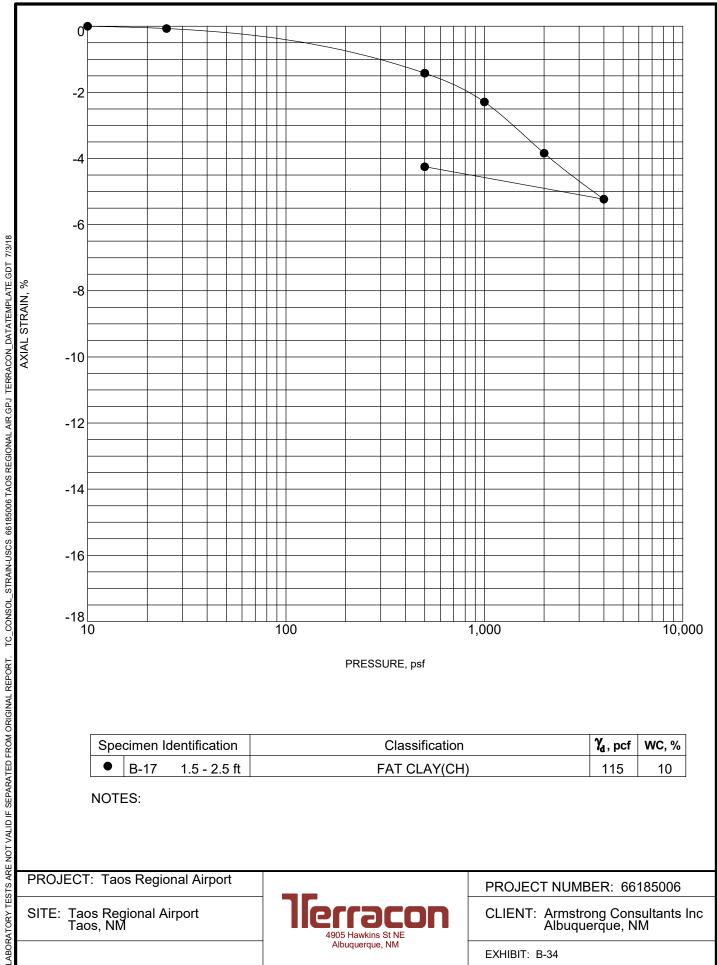
SITE: Taos Regional Airport Taos, NM

Albuquerque, NM

PROJECT NUMBER: 66185006

CLIENT: Armstrong Consultants Inc Albuquerque, NM

### **SWELL CONSOLIDATION TEST ASTM D2435**



PRESSU	JRE,	psf
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Specimen Identific	ation Classification	$\gamma_{\!_{f d}}$ , pcf	WC, %
● B-17 1.5 -	2.5 ft FAT CLAY(CH)	115	10

NOTES:

PROJECT: Taos Regional Airport

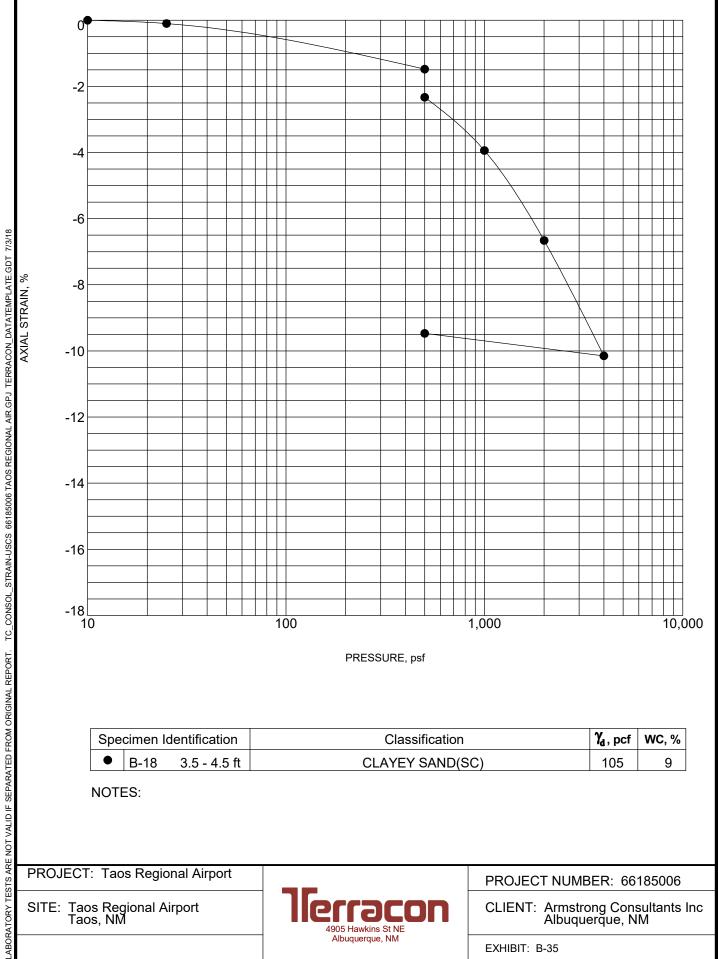
SITE: Taos Regional Airport Taos, NM



PROJECT NUMBER: 66185006

CLIENT: Armstrong Consultants Inc Albuquerque, NM

**ASTM D2435** 



PRESSURE,	nsf
FINESSUIVE.	DOL

Spe		pecimen Identification		Classification	$\gamma_d$ , pcf	WC, %
	•	B-18	3.5 - 4.5 ft	CLAYEY SAND(SC)	105	9

NOTES:

PROJECT: Taos Regional Airport

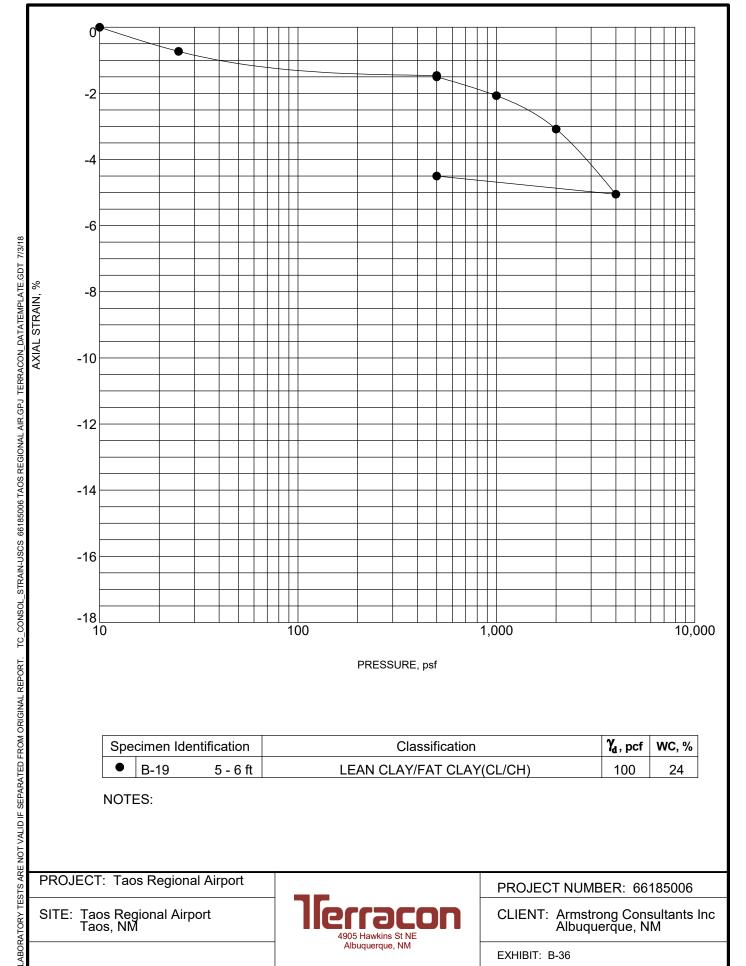
SITE: Taos Regional Airport Taos, NM



PROJECT NUMBER: 66185006

CLIENT: Armstrong Consultants Inc Albuquerque, NM





Spe	pecimen Identification		Classification	$\gamma_{d}$ , pcf	WC, %
•	B-19	5 - 6 ft	LEAN CLAY/FAT CLAY(CL/CH)	100	24

NOTES:

PROJECT: Taos Regional Airport

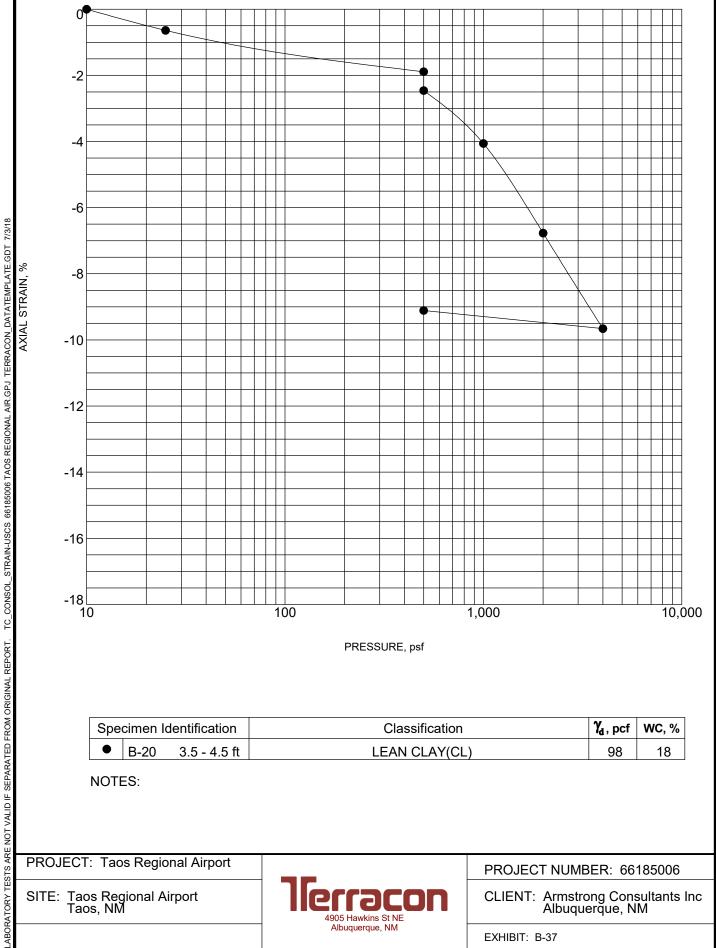
SITE: Taos Regional Airport Taos, NM



PROJECT NUMBER: 66185006

CLIENT: Armstrong Consultants Inc Albuquerque, NM





PRESSURE,	psf
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Specimen		dentification	Classification $\gamma_d$	$\gamma_{d}$ , pcf	WC, %
•	B-20	3.5 - 4.5 ft	LEAN CLAY(CL)	98	18

NOTES:

PROJECT: Taos Regional Airport

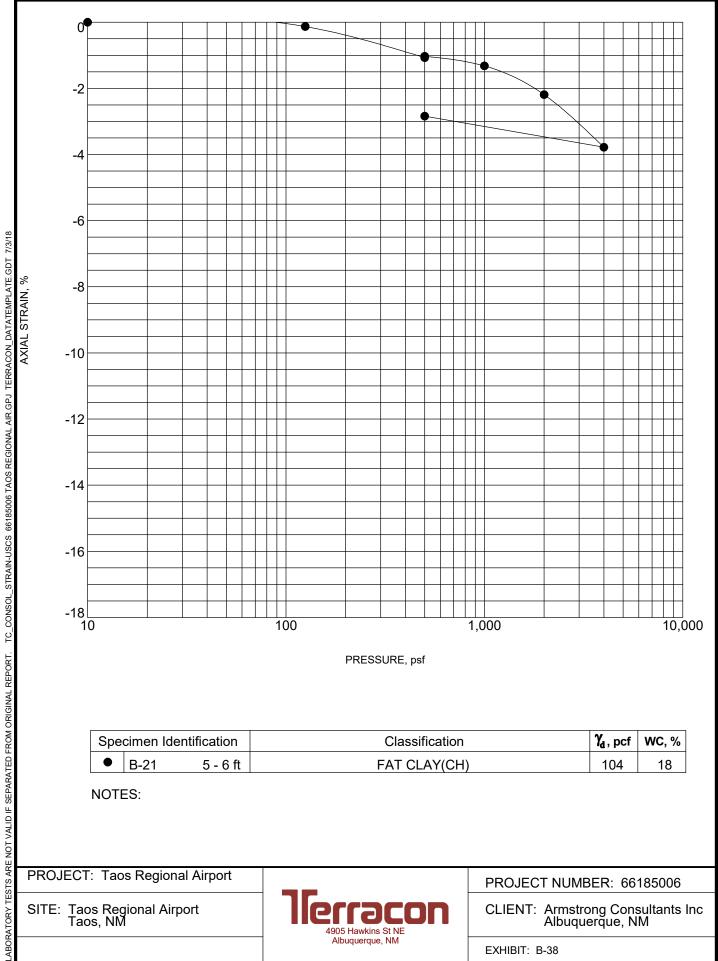
SITE: Taos Regional Airport Taos, NM



PROJECT NUMBER: 66185006

CLIENT: Armstrong Consultants Inc Albuquerque, NM

**ASTM D2435** 



Spe	ecimen Identification		Classification	$\gamma_d$ , pcf	WC, %
•	B-21	5 - 6 ft	FAT CLAY(CH)	104	18

NOTES:

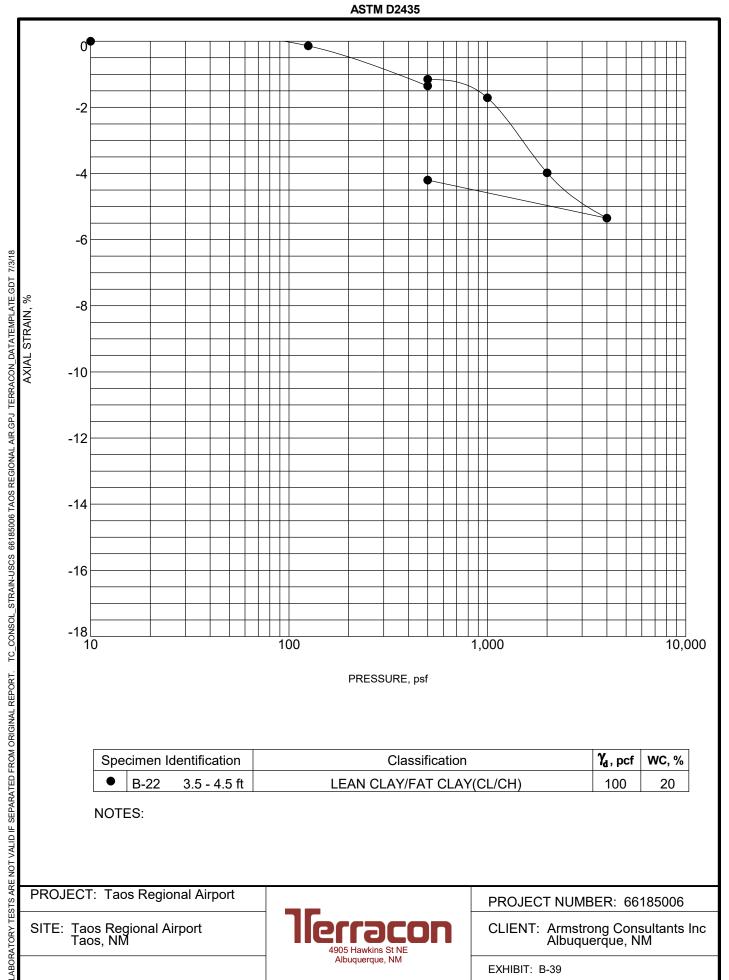
PROJECT: Taos Regional Airport

SITE: Taos Regional Airport Taos, NM



PROJECT NUMBER: 66185006

CLIENT: Armstrong Consultants Inc Albuquerque, NM



Spe		pecimen Identification		Classification	$\gamma_d$ , pcf	WC, %
	•	B-22	3.5 - 4.5 ft	LEAN CLAY/FAT CLAY(CL/CH)	100	20

NOTES:

PROJECT: Taos Regional Airport

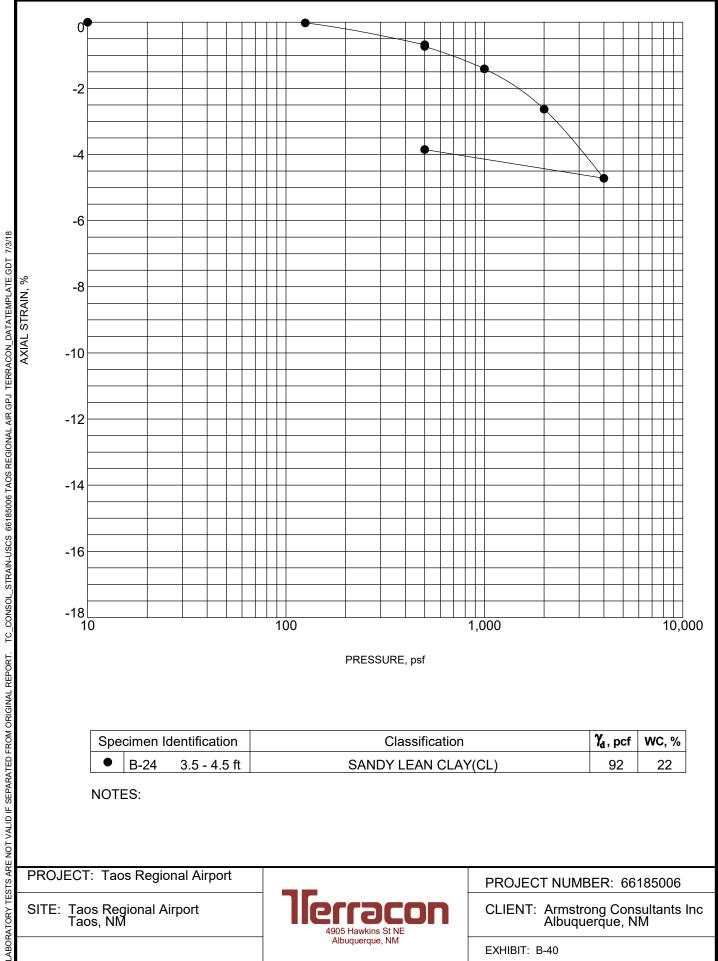
SITE: Taos Regional Airport Taos, NM



PROJECT NUMBER: 66185006

CLIENT: Armstrong Consultants Inc Albuquerque, NM

**ASTM D2435** 



PRESSU	JRE,	psf
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Specimen Identification		imon Idontification		$\gamma_d$ , pcf	WC, %	
	•   I	B-24	3.5 - 4.5 ft	SANDY LEAN CLAY(CL)	92	22

NOTES:

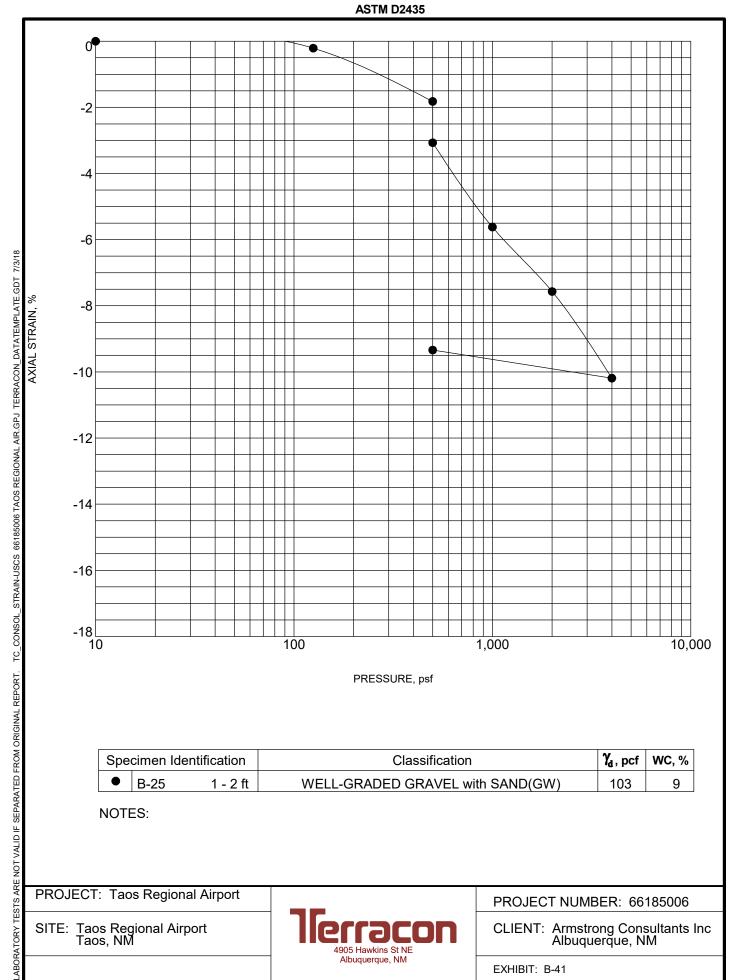
PROJECT: Taos Regional Airport

SITE: Taos Regional Airport Taos, NM



PROJECT NUMBER: 66185006

CLIENT: Armstrong Consultants Inc Albuquerque, NM



Specimen Identification		men Identification Classification		$\gamma_d$ , pcf	WC, %
•	B-25	1 - 2 ft	WELL-GRADED GRAVEL with SAND(GW)	103	9

NOTES:

PROJECT: Taos Regional Airport

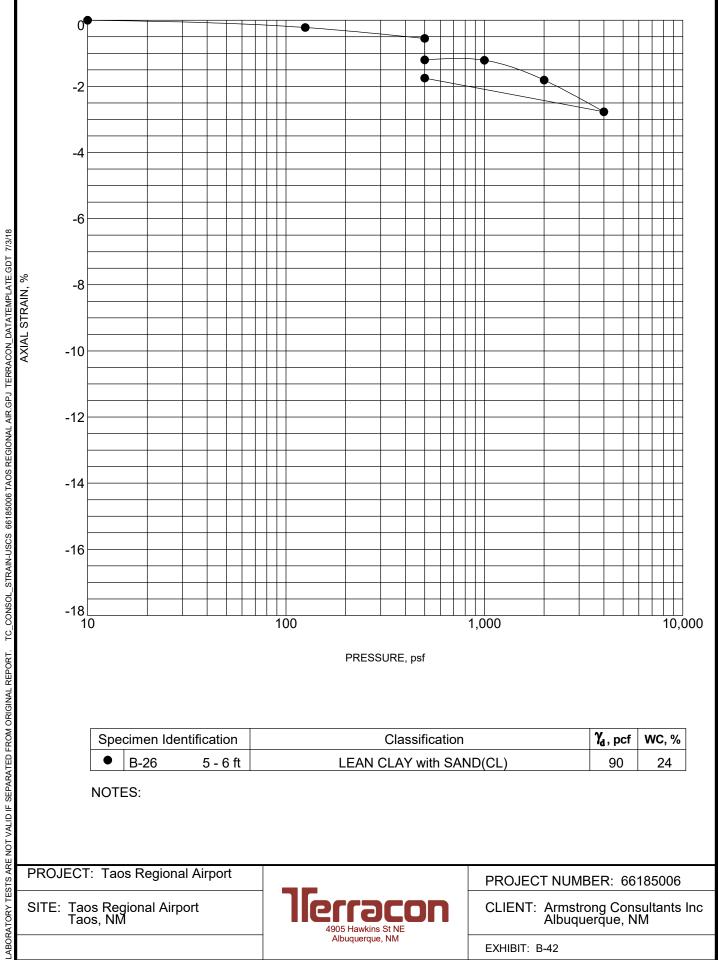
SITE: Taos Regional Airport Taos, NM



PROJECT NUMBER: 66185006

CLIENT: Armstrong Consultants Inc Albuquerque, NM

**ASTM D2435** 



Specimen Identification				$\gamma_d$ , pcf	WC, %
•	B-26	5 - 6 ft	LEAN CLAY with SAND(CL)	90	24

NOTES:

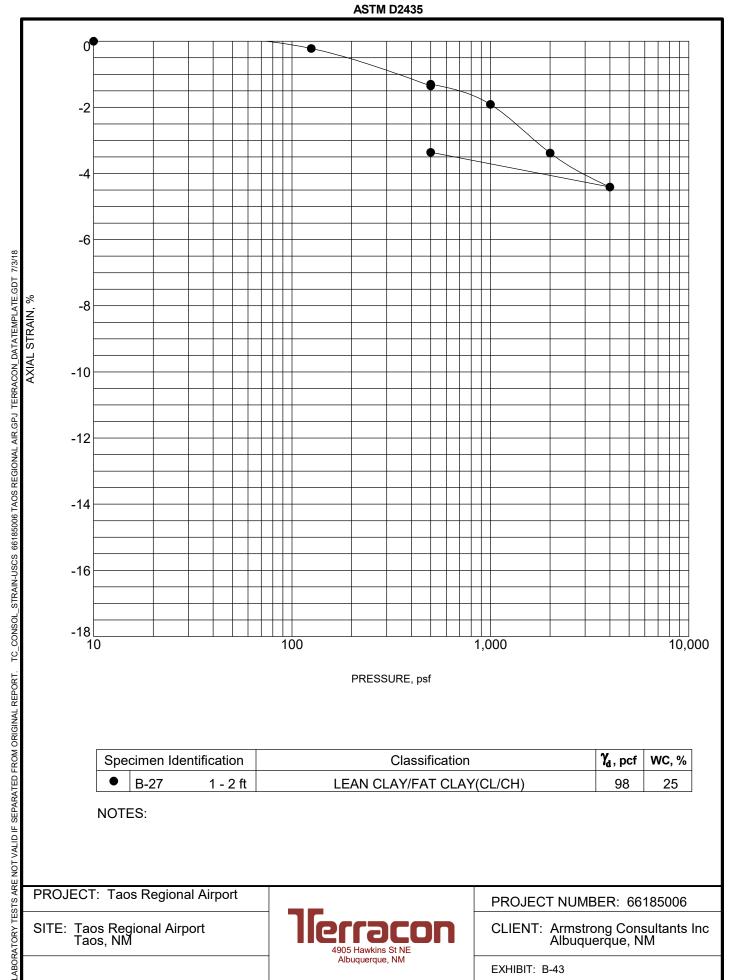
PROJECT: Taos Regional Airport

SITE: Taos Regional Airport Taos, NM



PROJECT NUMBER: 66185006

CLIENT: Armstrong Consultants Inc Albuquerque, NM



Specimen Identification		entification	Classification	$\gamma_d$ , pcf	WC, %
•	B-27	1 - 2 ft	LEAN CLAY/FAT CLAY(CL/CH)	98	25

NOTES:

PROJECT: Taos Regional Airport

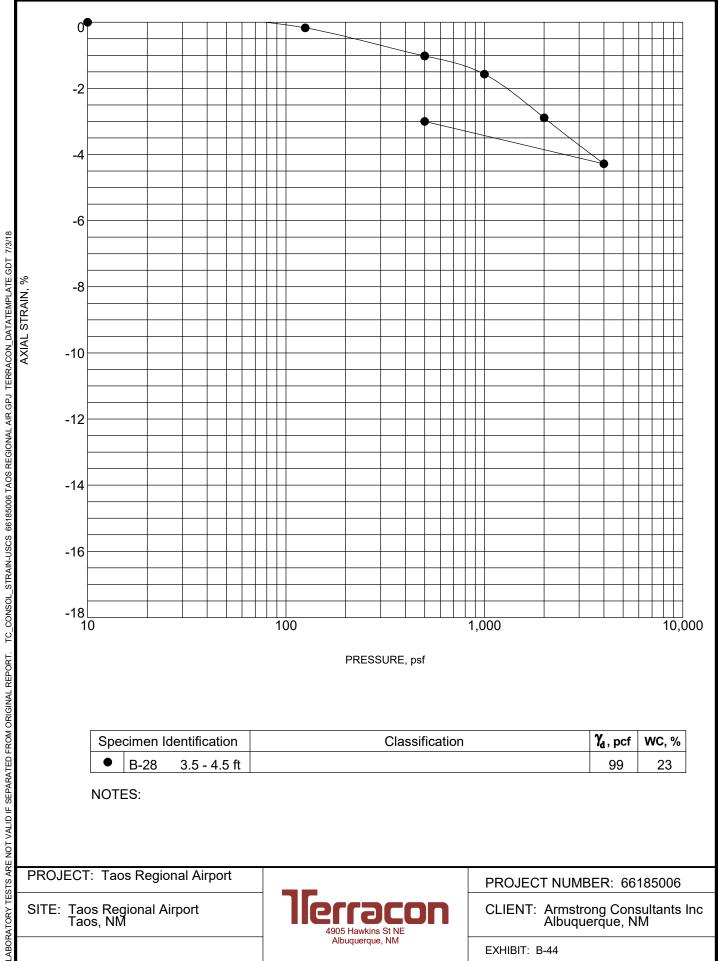
SITE: Taos Regional Airport Taos, NM



PROJECT NUMBER: 66185006

CLIENT: Armstrong Consultants Inc Albuquerque, NM

**ASTM D2435** 



PRESSURE, psf	PR	FSS	URF	nsf
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Spe	cimen lo	dentification	Classification	$\gamma_d$ , pcf	WC, %	
•	B-28	3.5 - 4.5 ft		99	23	

NOTES:

PROJECT: Taos Regional Airport

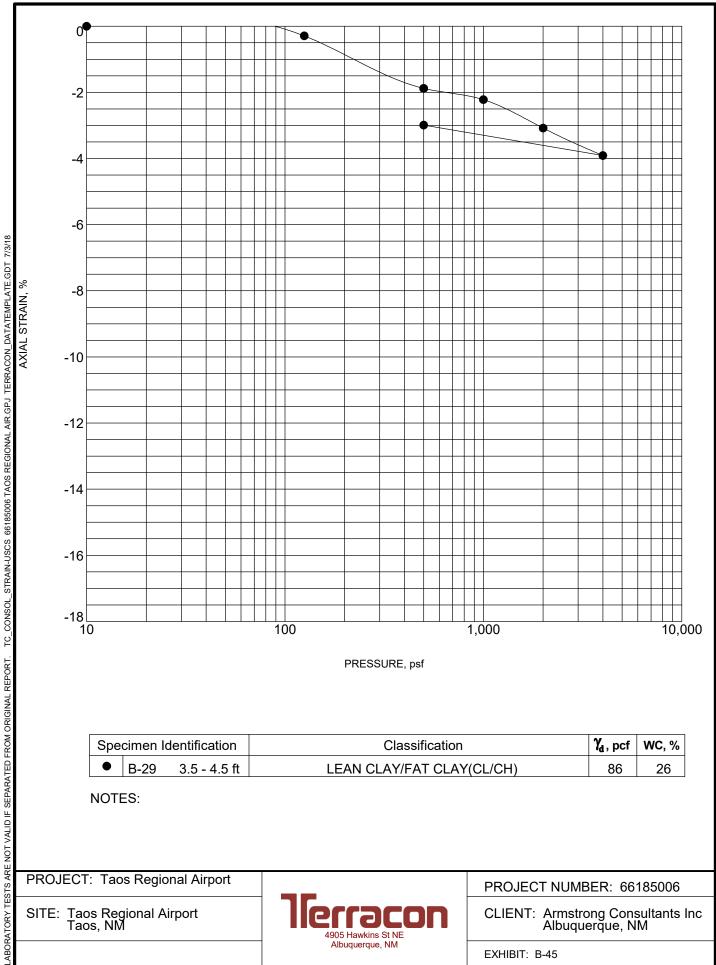
SITE: Taos Regional Airport Taos, NM



PROJECT NUMBER: 66185006

CLIENT: Armstrong Consultants Inc Albuquerque, NM

### **SWELL CONSOLIDATION TEST ASTM D2435**



PRESSURE, ps	st
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Specimen Identification		cimon Identification (Classification		$\gamma_{d}$ , pcf	WC, %
•	B-29	3.5 - 4.5 ft	LEAN CLAY/FAT CLAY(CL/CH)	86	26

NOTES:

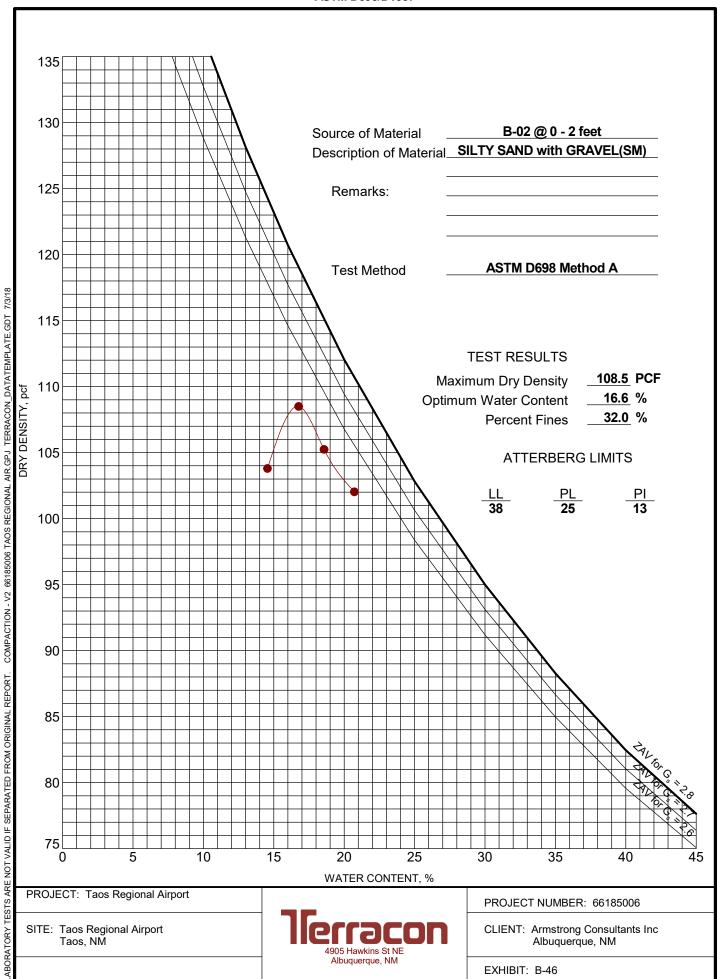
PROJECT: Taos Regional Airport

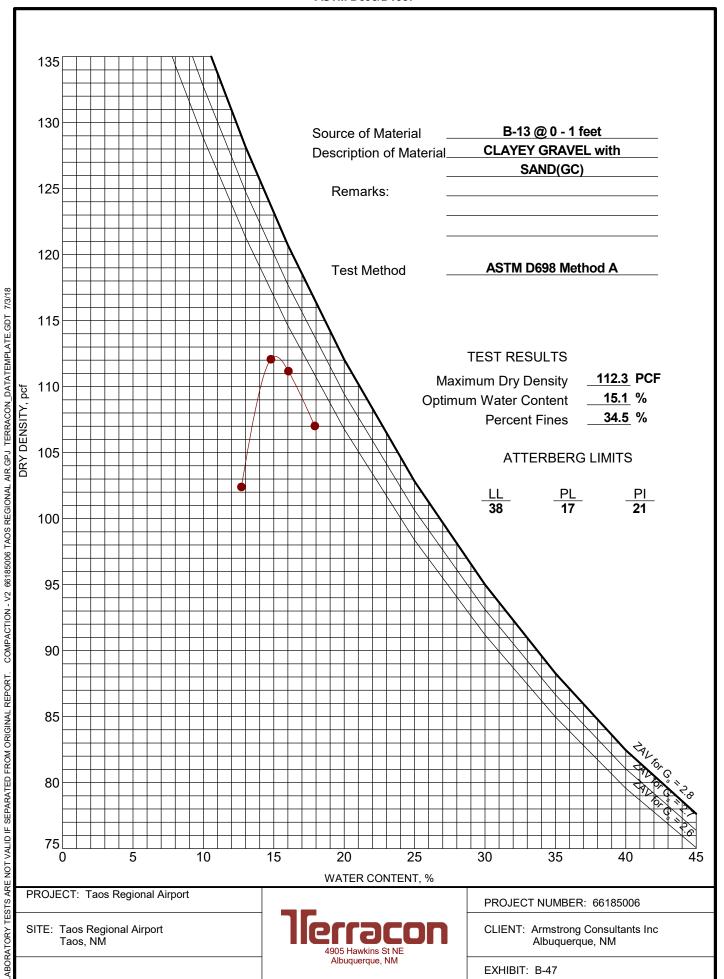
SITE: Taos Regional Airport Taos, NM

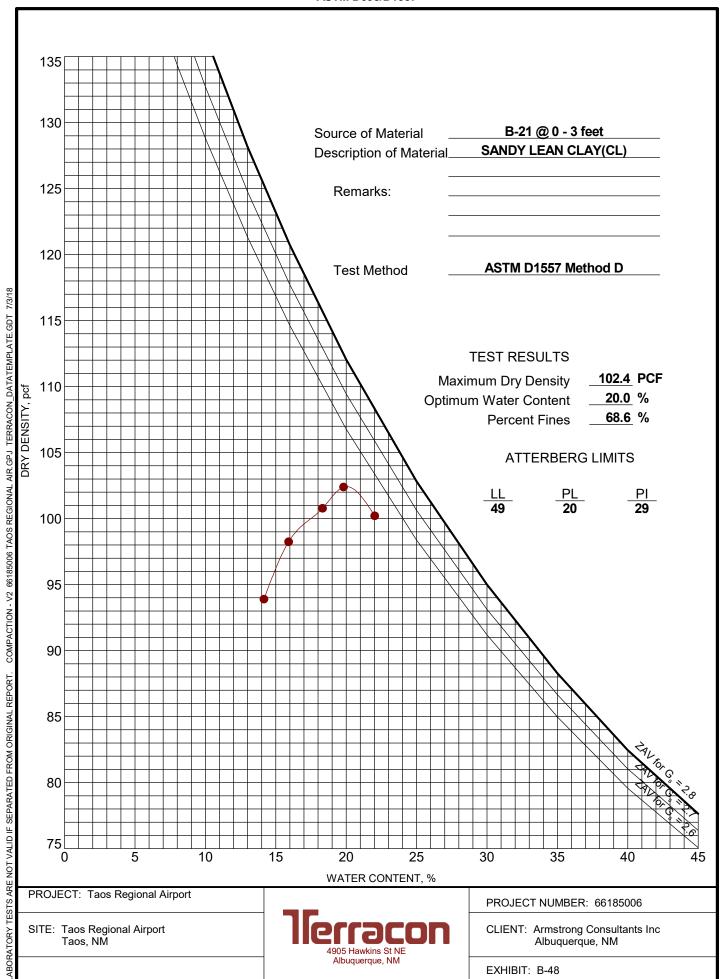


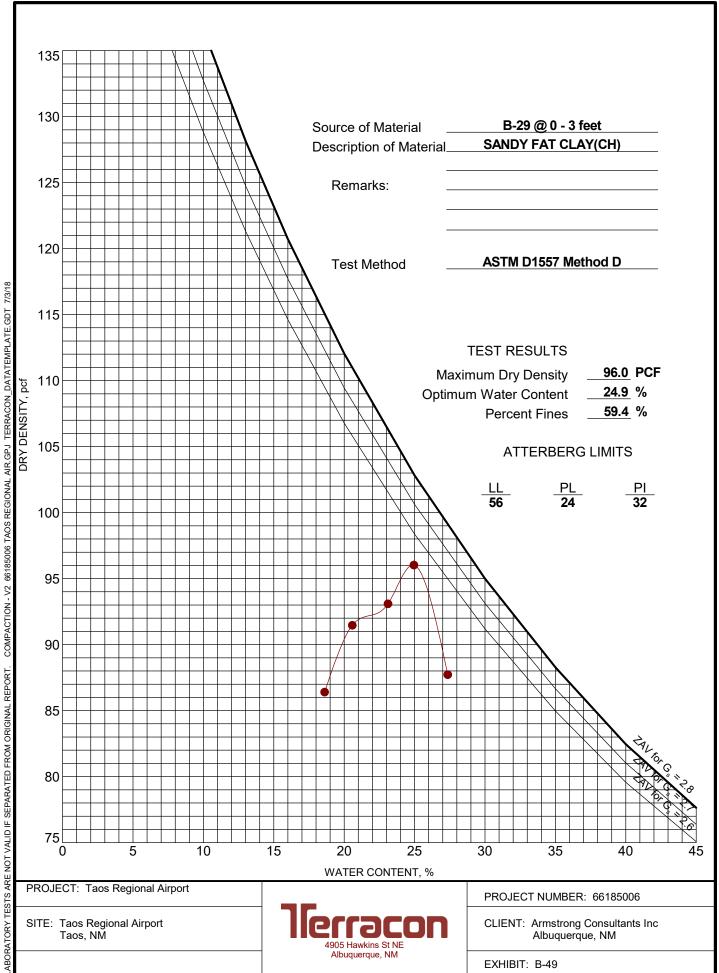
PROJECT NUMBER: 66185006

CLIENT: Armstrong Consultants Inc Albuquerque, NM











1579

Taos Regional Airport Albuquerque, NM Silty Sand w/Gravel (SM)

MATERIAL: Silty Sand w/Grave SAMPLE SOURCE: B-02 @ 0-2' (Bulk)

PROJECT: LOCATION:

WORK ORDER NO: LAB NO: 15

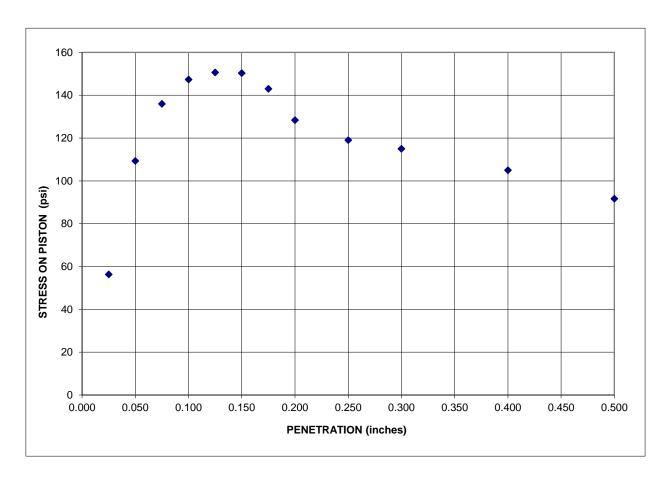
DATE SAMPLED:

# CBR(CALIFORNIA BEARING RATIO) OF LABORATORY-COMPACTED SOILS ASTM D1833 (SOAKED)

COMPACTION(%) COMPACTION METHOD	95 D1557 MODIFIED HAMMER 5 LIFTS @ 15 BLOWS/LIFT	PENETRATION CORRECTION CORRECTED CBR	0.100 0.00 14.7	0.200 0.00 8.6
SAMPLE CONDITION	SOAKED			
PERCENT SWELL	0.13%			
% MOISTURE (TOP 1")	17.3%	CBR (SOAKED)	15	
SURCHARGE WEIGHT	10 lbs			
% RETAINED 19mm SIEV	E 0%			

BEFORE SOAK

DRY DENSITY 102.9 lbs./cu.ft PERCENT MOISTURE 16.7 %





PROJECT: Taos Regional Airport LOCATION: Albuquerque, NM

MATERIAL: Clayey Gravel w/Sand (GC)

SAMPLE SOURCE: B-13 @ 0-1' (Bulk)

JOB NO: 66185006 WORK ORDER NO:

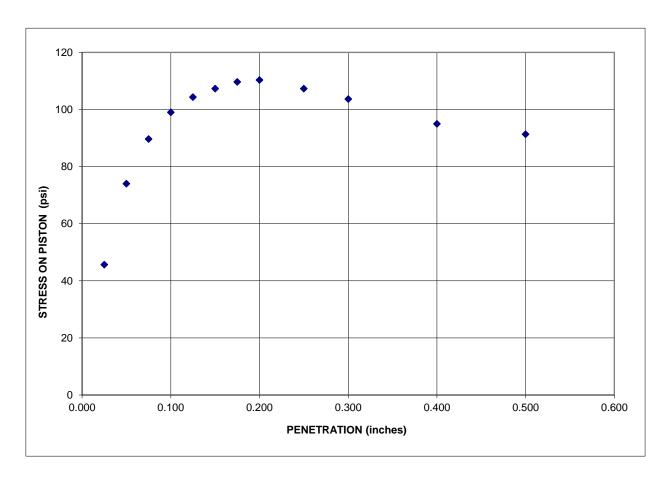
LAB NO: 1579 DATE SAMPLED:

# CBR(CALIFORNIA BEARING RATIO) OF LABORATORY-COMPACTED SOILS ASTM D1833 (SOAKED)

COMPACTION(%) COMPACTION METHOD	95 D1557 MODIFIED HAMMER 5 LIFTS @ 15 BLOWS/LIFT	PENETRATION CORRECTION CORRECTED CBR	0.100 0.00 9.9	0.200 0.00 7.4
SAMPLE CONDITION	SOAKED			
PERCENT SWELL	0.07%			
% MOISTURE (TOP 1")	16.4%	CBR (SOAKED)	9.9	
SURCHARGE WEIGHT	10 lbs			
% RETAINED 19mm SIEV	E 0%			

BEFORE SOAK

DRY DENSITY 106.9 lbs./cu.ft PERCENT MOISTURE 14.8 %





PROJECT: Taos Regional Airport
LOCATION: Albuquerque, NM
MATERIAL: Sandy Lean Clay (CL)
SAMPLE SOURCE: B-21 @ 0-3' (Bulk)

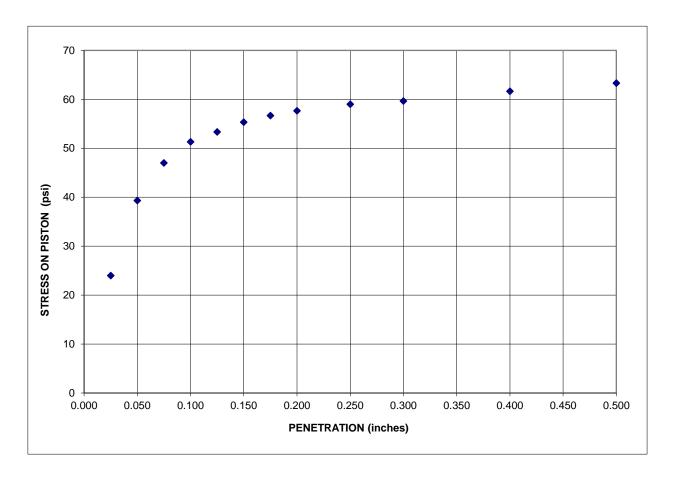
JOB NO: 66185006
WORK ORDER NO:
LAB NO: 1579
DATE SAMPLED:

# CBR(CALIFORNIA BEARING RATIO) OF LABORATORY-COMPACTED SOILS ASTM D1833 (SOAKED)

COMPACTION(%) 95 PENETRATION 0.100 0.200 COMPACTION METHOD D1557 MODIFIED HAMMER CORRECTION 0.00 0.00 5 LIFTS @ 10 BLOWS/LIFT CORRECTED CBR 5.1 3.8 SAMPLE CONDITION SOAKED PERCENT SWELL 0.24% % MOISTURE (TOP 1") 21.8% **CBR (SOAKED)** 5 SURCHARGE WEIGHT 10 lbs % RETAINED 19mm SIEVE 0%

**BEFORE SOAK** 

DRY DENSITY 97.2 lbs./cu.ft PERCENT MOISTURE 19.8 %





PROJECT: Taos Regional Airport
LOCATION: Albuquerque, NM
MATERIAL: Sandy Fat Clay (CH)
SAMPLE SOURCE: B-29 @ 0-3' (Bulk)

WORK ORDER NO: LAB NO: 1579 DATE SAMPLED:

8

# CBR(CALIFORNIA BEARING RATIO) OF LABORATORY-COMPACTED SOILS ASTM D1833 (SOAKED)

 COMPACTION(%)
 95
 PENETRATION
 0.100
 0.200

 COMPACTION METHOD
 D1557 MODIFIED HAMMER
 CORRECTION
 0.00
 0.00

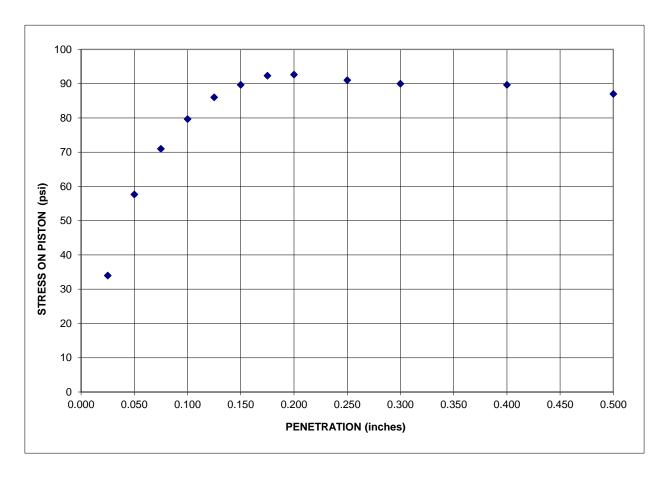
 5 LIFTS @ 9 BLOWS/LIFT
 CORRECTED CBR
 8.0
 6.2

**CBR (SOAKED)** 

SAMPLE CONDITION SOAKED
PERCENT SWELL 0.44%
% MOISTURE (TOP 1") 25.7%
SURCHARGE WEIGHT 10 lbs
% RETAINED 19mm SIEVE 0%

**BEFORE SOAK** 

DRY DENSITY 91.5 lbs./cu.ft PERCENT MOISTURE 24.6 %



**Summary of Laboratory Results** 

BORING ID	Depth	USCS Classification and Soil Description	Compressive Strength (tsf)	Liquid Limit	Plastic Limit	Plasticity Index	% <#200 Sieve	% Gravel	% Sand	% Silt	% Clay	Water Content (%)	Dry
B-01	1.5 - 3						66.2	3.0	30.8			21.5	
B-01	3.5 - 5						71.0	2.3	26.7			24.8	
B-01	5 - 6	FAT CLAY(CH)					22.7	34.7	42.5			22.7	84.
B-02	0 - 2	SILTY SAND with GRAVEL(SM)		38	25	13	32.0	32.7	35.3	20.2	11.8		
B-02	1.5 - 3						75.1	3.6	21.3			21.2	
B-02	3.5 - 5						49.9	14.9	35.2			22.8	
B-02	5 - 6	SANDY LEAN CLAY(CL)					43.8	9.9	41.8			30.6	71.
B-02	6.5												
B-02	10 - 11.5											13.5	
B-03	1.5 - 3						81.8	0.3	17.9			24.4	
B-03	3.5 - 4.5						61.7	0.7	37.6			24.8	90
B-03	5 - 6.5						37.0	16.3	46.7			32.2	
B-04	1.5 - 3						69.3	4.8	25.9			19.5	
B-04	3.5 - 5						77.5	2.7	19.8			20.6	
B-04	5 - 6	CLAYEY SAND(SC)					48.4	22.2	29.5			25.8	
B-05	1.5 - 3	OEXTET OF IND(OO)					10.0	60.4	29.7			4.7	
B-05	3.5 - 4.5	LEAN CLAY(CL)					72.7	1.0	26.3			20.5	98
B-05	5 - 6.5	LEAN CLAT(CL)					31.6	15.3	53.1			22.1	90
	10 - 11						31.0	15.5	33.1			15.9	
B-05							61.6	15.0	23.5			16.9	
B-06	1.5 - 3												
B-06	3.5 - 5	OLAVEY ORAYEL WHE CANDOO		50	40	20	77.2	3.5	19.3			18.9	
B-06	5 - 6	CLAYEY GRAVEL with SAND(GC)		50	18	32	44.2	39.3	16.5			13.5	
B-07	1.5 - 2.5	SANDY LEAN CLAY with  GRAVEL(CL)					69.9	8.6	21.5			21.0	97.
B-07	3.5 - 5	0.01722(02)					67.3	1.7	31.0			24.9	
B-07	5 - 6.5						56.9	0.4	42.7			25.4	
B-07	10 - 11.5						30.3	0.4	72.1			8.0	
B-08	1.5 - 3						37.4	34.9	27.7			10.3	
B-08	3.5 - 5						68.2	4.4	27.4			20.1	
B-08	5 - 6	LEAN CLAY/FAT CLAY(CL/CH)					75.0	0.4	24.6			23.2	77
		LEAN CLATIFAT CLAT(CL/CH)											- ' '
B-09	1.5 - 3 3.5 - 4.5	LEAN CLAVICAT CLAVICLICA					12.4	58.9	28.6			11.9	00
B-09	3.5 - 4.5 5 - 6.5	LEAN CLAY/FAT CLAY(CL/CH)					72.3	3.9	23.8			23.1	82
B-09 B-09							87.4	0.8	11.8			28.4	
B-09 B-10	10 - 11.5 1 - 2.5						57.2	4.6	38.2			18.9	
B-10	3.5 - 5	CANDY LEAN OLAWOLY					56.3	0.8	42.9			21.1	
B-10	5 - 6	SANDY LEAN CLAY(CL)					65.9	11.0	23.0			21.6	82
B-11	1 - 2.5	CANDY FAT OF AWOUR					36.4	29.2	34.4			11.5	-
B-11	3.5 - 4.5	SANDY FAT CLAY(CH)					59.7	20.5	19.7			14.3	98
B-11	5 - 6.5						71.3	5.8	22.9			18.0	
B-11	10 - 11.5								6			2.9	
B-12 PROJECT:	1 - 2.5 Taos Regi	ional Airport					17.1	47.9	35.0			6.0	
SITE: Taos		•	<b>Te</b> ri			חו		JECT N NT: Ari Ali		g Cor	nsultai		
				lawkins ( querque,					•				



**Summary of Laboratory Results** 

BORING		USCS Classification	Compressive	Liquid	Plastic	Plasticity	%	%	%	%	%	Sheet Water	Dry
ID	Depth	and Soil Description	Strength (tsf)	Limit	Limit	Index	<#200 Sieve	Gravel	Sand	Silt	Clay	Content (%)	Dens (pc
B-12	3.5 - 4.5	LEAN CLAY/FAT CLAY(CL/CH)					41.8	38.3	20.0			11.6	106
B-12	5 - 6.5						74.9	0.6	24.4			19.7	
B-13	0 - 1	CLAYEY GRAVEL with SAND(GC)		38	17	21	34.5	34.1	31.4	12.5	22.0		
B-13	0.5 - 2						22.8	33.7	43.5			7.8	
B-13	3.5 - 4.5	LEAN CLAY/FAT CLAY(CL/CH)					76.3	0.3	23.4			31.4	83
B-13	5 - 6.5						77.7	0.3	22.0			26.2	
B-13	10 - 11.5						13.4	61.1	25.5			5.4	
B-14	1 - 2						9.6	54.7	35.7			4.6	10
B-14	3.5 - 5						54.7	5.0	40.3			28.6	
B-14	4						75.5	1.6	22.9				
B-14	5 - 6.5											21.8	
B-15	1.5 - 3						46.3	24.6	29.1			13.6	
B-15	3.5 - 5						67.0	2.8	30.2			18.3	
B-15	6 - 7	LEAN CLAY with GRAVEL(CL)					46.6	22.1	31.4			12.3	91
B-15	10 - 11.42											2.5	
B-16	0 - 2												
B-16	1.5 - 3						89.8	1.2	8.9			22.7	
B-16	3.5 - 4.5	LEAN CLAY/FAT CLAY(CL/CH)					40.5	34.7	24.9			13.6	97
B-16	5 - 6.5	22.11.02.11.71.02.11(02.01.1)						J	20			20.4	"
B-16	10											20.4	
B-10	1.5 - 2.5	EAT CLAV(CH)					22.3	31.3	46.4			10.4	11
		FAT CLAY(CH)											11
B-17	3.5 - 5						67.8	1.4	30.8			22.0	
B-17	5 - 6.5						65.9	3.5	30.6			24.7	
B-17	10 - 11.5						20.0	40.0	00.5			3.0	
B-18	1.5 - 3						60.9	10.6	28.5			17.5	
B-18	3.5 - 4.5	CLAYEY SAND(SC)					25.9	32.9	41.2			9.0	10
B-18	5 - 6.5						35.7	41.9	22.4			11.1	
B-19	1 - 2.5						27.8	35.2	37.1			10.8	
B-19	3.5 - 5						50.8	9.4	39.7			20.4	
B-19	5 - 6	LEAN CLAY/FAT CLAY(CL/CH)					74.6	5.4	20.0			23.7	10
B-19	10 - 11.5											9.2	
B-20	1 - 2.5						71.9	4.2	23.8			20.2	
B-20	3.5 - 4.5	LEAN CLAY(CL)					71.3	0.2	28.5			17.9	97
B-20	5 - 6.5						66.2	1.4	32.4			20.1	
B-21	0 - 3	SANDY LEAN CLAY(CL)		49	20	29	68.6	0.0	31.4	38.1	30.6	14.2	
B-21	1 - 2.5						75.0	10.0	14.9			21.3	
B-21	3.5 - 5						83.4	1.5	15.1			23.8	
B-21	5 - 6	FAT CLAY(CH)					89.8	0.0	10.2			18.0	10
B-21	10 - 11.5											6.4	
B-22	1.5 - 3						19.6	31.8	48.6			12.3	
B-22	3.5 - 4.5	LEAN CLAY/FAT CLAY(CL/CH)					69.5	12.1	18.4			19.9	99
B-22	5 - 6.5						59.3	6.9	33.8			18.0	
PROJECT:	Taos Regi	onal Airport					PRO	JECT N	IUMBE	R: 6	61850	06	•
SITE: Taos Taos	Regional <i>i</i> s, NM	Airport				חו	CLIE	NT: Ar Al	mstron buquer			nts Inc	
				lawkins ( querque,				BIT: B					



**Summary of Laboratory Results** 

		1									-	Sheet	3 of 3
BORING ID	Depth	USCS Classification and Soil Description	Compressive Strength (tsf)	Liquid Limit	Plastic Limit	Plasticity Index	% <#200 Sieve	% Gravel	% Sand	% Silt	% Clay	Water Content (%)	Dry Dens (pcf
B-22	10 - 11.5											2.8	
B-23	1.5 - 2.5						83.3	0.1	16.6			26.1	95.1
B-23	3.5 - 5											21.1	
B-23	5 - 6.5						57.1	7.1	35.8			21.7	
B-23	10 - 11.5						66.5	1.3	32.2			6.7	
B-24	1 - 2.5						23.5	39.9	36.6			11.9	
B-24	3.5 - 4.5	SANDY LEAN CLAY(CL)					58.1	6.8	35.1			22.4	91.9
B-24	5 - 6.5						48.7	13.1	38.2			20.8	
B-25	1 - 2	WELL-GRADED GRAVEL with					2.6	52.5	44.9			9.2	102
		SAND(GW)											
B-25	3.5 - 5						59.8	8.4	31.8			19.0	
B-25	5 - 6.5						71.7	3.2	25.2			22.0	
B-25	10 - 11.5											5.2	
B-26	1.5 - 3						77.2	1.2	21.6			21.5	
B-26	3.5 - 5						68.2	4.0	27.8			20.8	
B-26	5 - 6	LEAN CLAY with SAND(CL)					73.6	0.2	26.3			24.2	89.
B-27	1 - 2	LEAN CLAY/FAT CLAY(CL/CH)					81.7	1.4	16.9			25.2	97.
B-27	3.5 - 5						78.5	0.3	21.2			25.5	
B-27	5 - 6.5						71.5	2.7	25.8			23.5	
B-27	10 - 11.5											2.9	
B-28	1.5 - 3						76.8	2.1	21.1			22.4	
B-28	3.5 - 4.5						84.0	0.0	16.0			23.5	99.
B-28	5 - 6.5						78.0	0.4	21.6			24.0	
B-28	6.5 - 8											26.2	
B-29	0 - 3	SANDY FAT CLAY(CH)		56	24	32	59.4	3.3	37.3	15.6	43.8	17.9	
B-29	1.5 - 3						60.3	3.5	36.2			22.8	
B-29	3.5 - 4.5	LEAN CLAY/FAT CLAY(CL/CH)					70.4	0.1	29.5			26.2	86.
B-29	5 - 6.5						58.9	0.3	40.9			28.8	
B-29	6.5 - 8											25.6	

PROJECT: Taos Regional Airport

SITE: Taos Regional Airport Taos, NM



PROJECT NUMBER: 66185006

CLIENT: Armstrong Consultants Inc

Albuquerque, NM



Hall Environmental Analysis Laboratory 4901 Hawkins NE Albuquerque, NM 87109 TEL: 505-345-3975 FAX: 505-345-4107 Website: www.hallenvironmental.com

May 25, 2018

Mike Anderson
Terracon
4905 Hawkins, NE
Albuquerque, NM 87109
TEL: (505) 797-4287

TEL: (505) 797-4287 FAX (505) 797-4288

RE: Taos Regional Airport OrderNo.: 1805A18

#### Dear Mike Anderson:

Hall Environmental Analysis Laboratory received 2 sample(s) on 5/17/2018 for the analyses presented in the following report.

These were analyzed according to EPA procedures or equivalent. To access our accredited tests please go to <a href="www.hallenvironmental.com">www.hallenvironmental.com</a> or the state specific web sites. In order to properly interpret your results, it is imperative that you review this report in its entirety. See the sample checklist and/or the Chain of Custody for information regarding the sample receipt temperature and preservation. Data qualifiers or a narrative will be provided if the sample analysis or analytical quality control parameters require a flag. When necessary, data qualifiers are provided on both the sample analysis report and the QC summary report, both sections should be reviewed. All samples are reported, as received, unless otherwise indicated. Lab measurement of analytes considered field parameters that require analysis within 15 minutes of sampling such as pH and residual chlorine are qualified as being analyzed outside of the recommended holding time.

Please don't hesitate to contact HEAL for any additional information or clarifications.

ADHS Cert #AZ0682 -- NMED-DWB Cert #NM9425 -- NMED-Micro Cert #NM0901

Sincerely,

Andy Freeman

Laboratory Manager

andyl

4901 Hawkins NE

Albuquerque, NM 87109

# **Analytical Report**Lab Order **1805A18**

Date Reported: 5/25/2018

## Hall Environmental Analysis Laboratory, Inc.

CLIENT: Terracon Client Sample ID: B-02 (Bulk)

**Project:** Taos Regional Airport
 Collection Date: 5/17/2018 2:00:00 PM

 **Lab ID:** 1805A18-001
 Matrix: SOIL
 Received Date: 5/17/2018 2:10:00 PM

Analyses	Result	PQL Qua	l Units	DF	Date Analyzed	Batch
EPA METHOD 300.0: ANIONS					Analy	st: MRA
Sulfate	3100	75	mg/Kg	50	5/22/2018 7:14:27 PI	M 38246

Refer to the QC Summary report and sample login checklist for flagged QC data and preservation information.

of 3
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# **Analytical Report**Lab Order **1805A18**

Date Reported: 5/25/2018

## Hall Environmental Analysis Laboratory, Inc.

CLIENT: Terracon Client Sample ID: B-13 (Bulk)

**Project:** Taos Regional Airport **Collection Date:** 5/17/2018 2:00:00 PM

**Lab ID:** 1805A18-002 **Matrix:** SOIL **Received Date:** 5/17/2018 2:10:00 PM

Analyses	Result	PQL Qu	al Units	DF	Date Analyzed	Batch
EPA METHOD 300.0: ANIONS					Analy	/st: MRA
Sulfate	100	1.5	mg/Kg	1	5/22/2018 4:29:43 A	M 38246

Refer to the QC Summary report and sample login checklist for flagged QC data and preservation information.

*	Value exceeds Maximum Contaminant Level.	В	Analyte detected in the associated Method Blank
D	Sample Diluted Due to Matrix	E	Value above quantitation range
Н	Holding times for preparation or analysis exceeded	J	Analyte detected below quantitation limits Page 2 of 3
ND	Not Detected at the Reporting Limit	P	Sample pH Not In Range
PQL	Practical Quanitative Limit	RL	Reporting Detection Limit
S	% Recovery outside of range due to dilution or matrix	W	Sample container temperature is out of limit as specified
	D H ND	D Sample Diluted Due to Matrix H Holding times for preparation or analysis exceeded ND Not Detected at the Reporting Limit PQL Practical Quanitative Limit	D     Sample Diluted Due to Matrix     E       H     Holding times for preparation or analysis exceeded     J       ND     Not Detected at the Reporting Limit     P       PQL     Practical Quantitative Limit     RL

## **QC SUMMARY REPORT**

### Hall Environmental Analysis Laboratory, Inc.

WO#: **1805A18** 

25-May-18

Client: Terracon

**Project:** Taos Regional Airport

Sample ID MB-38246 SampType: mblk TestCode: EPA Method 300.0: Anions

Client ID: PBS Batch ID: 38246 RunNo: 51408

Prep Date: 5/21/2018 Analysis Date: 5/21/2018 SeqNo: 1673973 Units: mg/Kg

Analyte Result PQL SPK value SPK Ref Val %REC LowLimit HighLimit %RPD RPDLimit Qual

Sulfate ND 1.5

Sample ID LCS-38246 SampType: Ics TestCode: EPA Method 300.0: Anions

Client ID: LCSS Batch ID: 38246 RunNo: 51408

Prep Date: 5/21/2018 Analysis Date: 5/21/2018 SeqNo: 1673974 Units: mg/Kg

Analyte Result PQL SPK value SPK Ref Val %REC LowLimit HighLimit %RPD RPDLimit Qual

Sulfate 28 1.5 30.00 0 92.7 90 110

#### Qualifiers:

\* Value exceeds Maximum Contaminant Level.

D Sample Diluted Due to Matrix

H Holding times for preparation or analysis exceeded

ND Not Detected at the Reporting Limit

PQL Practical Quanitative Limit

S % Recovery outside of range due to dilution or matrix

B Analyte detected in the associated Method Blank

E Value above quantitation range

J Analyte detected below quantitation limits

P Sample pH Not In Range

RL Reporting Detection Limit

W Sample container temperature is out of limit as specified

Page 3 of 3



### Hall Environmental Analysis Laboratory 4901 Hawkins NE Albuquerque, NM 87109

TEL: 505-345-3975 FAX: 505-345-4107 Website: www.hallenvironmental.com

# Sample Log-In Check List

Client Name: TER-Alb Work Order N	lumber: 1805A18		RoptNo: 1	
Received By: Anne Thorne 5/17/2018 2:10	00 PM	anne A.		
Completed By: Michelle Garcia 5/17/2018 3:53.	21 PM	Michelle Ge	0424	The second secon
Reviewed By: 65/18/18			Labeled	by 56 05/
Chain of Custody		1		
1. Is Chain of Custody complete?	Yes 🗹	No 🗌	Not Present	
2. How was the sample delivered?	<u>Client</u>			
<u>Log In</u>			<u>.</u>	
Was an attempt made to cool the samples?	Yes 🗹	No 🗌	NA 📙 🦼	
4. Were all samples received at a temperature of >0° C to 6.0°C		No 🗹	na 🗆	
E 0 1/11: 12: 12: 12:	not required.			•
5. Sample(s) in proper container(s)?	Yes 🗸	No L		
6. Sufficient sample volume for indicated test(s)?	Yes 🗹	No 🗌	e e e e e e e e e e e e e e e e e e e	
7. Are samples (except VOA and ONG) properly preserved?	Yes 🗹	No 🗆		
8. Was preservative added to bottles?	Yes 🗌	No 🗹	NA 🗆	/ /
0.404	., 🗖	N. []		/ / //8
9. VOA vials have zero headspace?	Yes ∐	No 🗔	No VOA Vials ✓	/ 118/
10. Were any sample containers received broken?	Yes 🗀	No 🗹 🗍	# of preserved	d5/1
11. Does paperwork match bottle labels? (Note discrepancies on chain of custody)	Yes 🗹	No 🗆	bottles checked for pH:	Maless noted)
12. Are matrices correctly identified on Chain of Custody?	Yes 🔽	No 🗆	Adjusted?	5
13, Is it clear what analyses were requested?	Yes <b>✓</b>	No 🗆	/ 7	
14. Were all holding times able to be met?	Yes 🔽	No 🗆	Checked by:	
(If no, notify customer for authorization.)		<u>.</u>		· · · · · · · · · · · · · · · · · · ·
Special Handling (if applicable)				
15. Was client notified of all discrepancies with this order?	Yes	No 🗌	NA 🗹	
Person Notified: D	ate:			-
By Whom: V	ia: eMail	Phone   Fax	☐ In Person	
: Regarding:				
Client Instructions:	and the second of the second o	100 man, -1, -1, -1, -1, -1, -1, -1, -1, -1, -1	THE TAX THE STATE OF THE STATE AND	•
16. Additional remarks:		HIIIW-A H	*	
17. Cooler Information		-	·.	

Chain	of-Cu	Chain-of-Custody Record	Turn-Around Time:				H		2	HALL FNVTRONMENTAL	2 2 2	2 11	Y.E	
Client: TERR	Acon	TERRACON CONSULTANTS, THE	☑Standard	□ Rush			A	\     	ANALYSIS		LABORATORY	Ş	0.0	, <u>}</u>
			;;	4 20 20			M/M	.haller	vironn		mos	! !	i )	1
Mailing Address:	5064	5 thusland ST NE	1405 KE	KEGIONATU III KOM	49	01 Ha	4901 Hawkins NE		ənbnqı	- Albuquerque, NM 87109	VM 87	601		
			Project #:	11105016	F	el. 505	Tel. 505-345-3975	175	Fax	505-345-4107	5-4107			
Phone #: 5	-505	757-4287	9					Ans	lysis	Analysis Request	st			
email or Fax#:	505-	332h-25E	Project Manager:						(°C	•				
AA/QC Package: WStandard		☐ Level 4 (Full Validation)	M:1EF A	Anoensau				(SMI	§'⁺0d	bcB₁≅				
Accreditation			Sampler:	1		) DR		S 02	10 <sup>5</sup> 'l	Z808				(1
□ NELAP	□ Other		On Ice: Tes	Z No		90				8 / 8	(A			/ 10
☐ EDD (Type)_			Sample Temperature:	e: - ( <b>)</b> (2) 5 · 3		(G <del>l</del>								) (Y
Date Time	Matrix Sove	Sample Request ID	Container Preserva Type and # Type	Preservative HEAL No. Type //KN5A/K	TM + X3T8 TM + X3T8	86108 H9T	TPH (Metho	PAH's (831) RCRA 8 Me	O,7) snoinA	8081 Pestic 40V) B0828	imə2) 0728			Air Bubbles
70 /1	$\rightarrow$	18-02 (Bune)		100					/					
		B-13 (BUIL)		200					1					
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Jate: Time: 1 17/18 2:10pm	Relinquished by:	Son The	Received by:	Date Time 7	Remarks	:S								
Date: Time:	Relinquished by:	ed by:	Received by C	Date Time										
If necessary,	samples sub	If necessary, samples submitted to Hall Environmental may be subcontracted to other accredited laboratories. This serves as notice of this possibility. Any sub-contracted data will be clearly notated on the analytical report	intracted to other accredited is	aboratories. This serves as notice of this I	ossibility.	Any sub-	contracted	data will	be clearly	/ notated	on the an	alytical re	sport.	



Hall Environmental Analysis Laboratory 4901 Hawkins NE Albuquerque, NM 87109 TEL: 505-345-3975 FAX: 505-345-4107 Website: www.hallenvironmental.com

May 14, 2018

Mike Anderson
Terracon
4905 Hawkins, NE
Albuquerque, NM 87109
TEL: (505) 797-4287

TEL: (505) 797-4287 FAX (505) 797-4288

RE: Taos Regional Airport OrderNo.: 1805268

#### Dear Mike Anderson:

Hall Environmental Analysis Laboratory received 2 sample(s) on 5/3/2018 for the analyses presented in the following report.

These were analyzed according to EPA procedures or equivalent. To access our accredited tests please go to <a href="www.hallenvironmental.com">www.hallenvironmental.com</a> or the state specific web sites. In order to properly interpret your results, it is imperative that you review this report in its entirety. See the sample checklist and/or the Chain of Custody for information regarding the sample receipt temperature and preservation. Data qualifiers or a narrative will be provided if the sample analysis or analytical quality control parameters require a flag. When necessary, data qualifiers are provided on both the sample analysis report and the QC summary report, both sections should be reviewed. All samples are reported, as received, unless otherwise indicated. Lab measurement of analytes considered field parameters that require analysis within 15 minutes of sampling such as pH and residual chlorine are qualified as being analyzed outside of the recommended holding time.

Please don't hesitate to contact HEAL for any additional information or clarifications.

ADHS Cert #AZ0682 -- NMED-DWB Cert #NM9425 -- NMED-Micro Cert #NM0190

Sincerely,

Andy Freeman

Laboratory Manager

andyl

4901 Hawkins NE

Albuquerque, NM 87109

### **Analytical Report**

#### Lab Order **1805268**

## Hall Environmental Analysis Laboratory, Inc.

Date Reported: 5/14/2018

CLIENT: TerraconClient Sample ID: B-21 @ 0-3Project: Taos Regional AirportCollection Date: 5/1/2018

**Lab ID:** 1805268-001 **Matrix:** SOIL **Received Date:** 5/3/2018 2:35:00 PM

Analyses	Result	PQL Qu	al Units	DF	Date Analyzed	Batch
EPA METHOD 300.0: ANIONS					Analy	st: MRA
Sulfate	170	30	mg/Kg	20	5/8/2018 4:29:15 PM	37984

Refer to the QC Summary report and sample login checklist for flagged QC data and preservation information.

tected in the associated Method Blank
iceled in the associated Method Diank
e quantitation range
tected below quantitation limits Page 1 of 3
Not In Range
Detection Limit
ntainer temperature is out of limit as specified
[

### **Analytical Report**

#### Lab Order **1805268**

## Hall Environmental Analysis Laboratory, Inc.

Date Reported: 5/14/2018

CLIENT: Terracon

Client Sample ID: B-29 @ 0-3

Project: Taos Regional Airport

Collection Date: 5/1/2018

**Lab ID:** 1805268-002 **Matrix:** SOIL **Received Date:** 5/3/2018 2:35:00 PM

Analyses	Result	PQL Qua	al Units	DF Date Analyzed	Batch
EPA METHOD 300.0: ANIONS				Anal	yst: MRA
Sulfate	3300	75	mg/Kg	50 5/10/2018 8:03:17 P	M 38010

Refer to the QC Summary report and sample login checklist for flagged QC data and preservation information.

Qualifiers:	*	Value exceeds Maximum Contaminant Level.	В	Analyte detected in the associated Method Blank
	D	Sample Diluted Due to Matrix	E	Value above quantitation range
	Н	Holding times for preparation or analysis exceeded	J	Analyte detected below quantitation limits Page 2 of 3
	ND	Not Detected at the Reporting Limit	P	Sample pH Not In Range
	PQL	Practical Quanitative Limit	RL	Reporting Detection Limit
	S	% Recovery outside of range due to dilution or matrix	W	Sample container temperature is out of limit as specified

### **QC SUMMARY REPORT**

### Hall Environmental Analysis Laboratory, Inc.

WO#: **1805268** 

14-May-18

Client: Terracon

**Project:** Taos Regional Airport

Sample ID MB-37984 SampType: mblk TestCode: EPA Method 300.0: Anions

Client ID: PBS Batch ID: 37984 RunNo: 51112

Prep Date: 5/8/2018 Analysis Date: 5/8/2018 SeqNo: 1661424 Units: mg/Kg

Analyte Result PQL SPK value SPK Ref Val %REC LowLimit HighLimit %RPD RPDLimit Qual

Sulfate ND 1.5

Sample ID LCS-37984 SampType: Ics TestCode: EPA Method 300.0: Anions

Client ID: LCSS Batch ID: 37984 RunNo: 51112

Prep Date: 5/8/2018 Analysis Date: 5/8/2018 SeqNo: 1661425 Units: mg/Kg

Analyte Result PQL SPK value SPK Ref Val %REC LowLimit HighLimit %RPD RPDLimit Qual

Sulfate 29 1.5 30.00 0 96.0 90 110

Sample ID MB-38010 SampType: mblk TestCode: EPA Method 300.0: Anions

Client ID: PBS Batch ID: 38010 RunNo: 51146

Prep Date: 5/9/2018 Analysis Date: 5/9/2018 SeqNo: 1662412 Units: mg/Kg

Analyte Result PQL SPK value SPK Ref Val %REC LowLimit HighLimit %RPD RPDLimit Qual

Sulfate ND 1.5

Sample ID LCS-38010 SampType: Ics TestCode: EPA Method 300.0: Anions

Client ID: LCSS Batch ID: 38010 RunNo: 51146

Prep Date: 5/9/2018 Analysis Date: 5/9/2018 SeqNo: 1662413 Units: mg/Kg

Analyte Result PQL SPK value SPK Ref Val %REC LowLimit HighLimit %RPD RPDLimit Qual

Sulfate 28 1.5 30.00 0 93.6 90 110

#### Qualifiers:

\* Value exceeds Maximum Contaminant Level.

D Sample Diluted Due to Matrix

H Holding times for preparation or analysis exceeded

ND Not Detected at the Reporting Limit

PQL Practical Quanitative Limit

S % Recovery outside of range due to dilution or matrix

B Analyte detected in the associated Method Blank

E Value above quantitation range

J Analyte detected below quantitation limits

P Sample pH Not In Range

RL Reporting Detection Limit

W Sample container temperature is out of limit as specified

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Hall Environmental Analysis Laboratory 4901 Hawkins NE Albuquerque, NM 87109 TEL: 505-345-3975 FAX: 505-345-4107

Website: www.hallenvironmental.com

# Sample Log-In Check List

С	lient Name:	TER-Alb		Worl	k Order Num	ber: 180	5268				RoptN	No 1	- 13
Re	oce ved By:	Erin Mele	endrez	5/3/20	18 2:35:00 F	РМ		u	az Z	-			
Co	mpleted By:	Ashley G	allegos	5/4/20	18 8:57:57 A	M		A	€.				
Re	eviewed By:	Iu	0	5/4/1			oe l	ecl	be	1: E	NH.		
Ch	ain of Cus	tody							8				
700	Is Chain of C		lete?			Yes	~	No		Not F	Present _		
2.	How was the	sample deliv	vered?			Clie	nt.						
1.	og In												
227	Was an attern	npt made to	cool the samp	oles?		Yes		No	<b>V</b>		NA 🗆		
4.	Were all samp	ales received	l at a tempera	ature of >0° C	to 6.0°C	Yes	П	: No	<b>v</b>		NA 🗆	ļ.	
5.	Sample(s) in p	proper conta	iner(s)?			Yes	V	: No					
6.	Sufficient sam	ple volume t	or indicated t	est(s)?		Yes	~	No					
7.	Are samples (	except VOA	and ONG) pr	operly preserv	red?	Yes	<b>Y</b>	No					
8. 1	Was preserva	tive added to	bottles?			Yes		No	~		NA 🗌		
9. 1	VOA vials hav	e zero head:	space?			Yes	<b>v</b>	No		No VOA	Vials		
10.	Were any san	nple containe	ers received t	oroken?		Yes		No	~	# of pre	served	ONTHE	
	Does paperwo Note discrepa			ò		Yes	V	No		bottles of for pH	checked	unless note	d)
12.	Are matrices o	correctly iden	tified on Cha	n of Custody?		Yes	~	No		A	dina 645	/	
13.1	s it clear what	t analyses w	ere requested	17		Yes	~	No		67	3		
	Nere all holdin					Yes	~	No		Ch	ecked by		
	cial Handli								/				
			(1) (1) (1) (1) (1) (1) (1) (1) (1)	with this order	?	Yes		No			NA 🗹		
	Person	Notified			Date								
	By Who	m:			Via	☐ eM	ail _	Phone [	Fax	In Pe	rson		
	Regardi	ng:											
	Client In	structions:											
16.	Additional ren	marks:											
17.	Cooler Inform	mation											
	Cooler No	Temp ℃	Condition	Seal Intact	Seal No	Seal D	ate	Signed	Ву				
	[1	22.8	Good	Not Present						ļ.			

Chain-of-Custody Record	Turn-Around Time:	HALLENVIDONMENTAL
Slient Temporen INC.	XStandard □ Rush	ANALYSIS LABORATORY
	Project Name:	www.hallenvironmental.com
Aailing Address 4 CE Hawking	Tack Realonal Lilbert	4901 Hawk
		Tel. 505-345-3975
4.4287	(20185006	Anal
mail or Fax#:	Project Manager:	30)
A/QC Package:   Standard   Level 4 (Full Validation)	Wike Lirenan	OS / WIE
ccreditation	Sampler	(1) (1) (1) (1) (1) (1) (1) (1) (1) (1)
□ NELAP □ Other	On Ice: ☐ Yes X No	O5 81 .40 .40 .50 .40 .40 .40 .40 .40 .40 .40 .40 .40 .4
1 EDD (Type)	Sample Temperature: 22 - 8	(GE See See See See See See See See See Se
Date Time Matrix Sample Request ID	Container Preservative HEAL No.	BTEX + MT BTEX + MT TPH 80158 TPH (Metho EDB (Metho RCRA 8 Me Anions (F,C B081 Pestic Anions (F,C B250B (VOA B260B (VOA B260B (VOA
18/8/8-04-03	\$	
V B. e.29		
B-2100-3	100-	8) /2°V
B.29.00.3	200-	XXX
3.18 Time: Relinquished by:	Received by.    Date Time	Remarks:
if nanocessure committee to inviting to like I Frantmental may be supported to	ontracted to other appredited laboratories. This series as notice of this	other ancredited laboratories. This serves as notice of this possibility. Any sub-contracted data will be clearly notated on the analytical penotic
Il lideasaary, samples seeminger or an entire minimum and an entire	NATIONAL SECTION OF THE SECURITIES ASSESSED TO SECURITIES ASSESSED T	and a more frame and the consequence of the same and the